From:	Kathryn Saunders
Sent:	Wed, 1 Jul 2020 10:09:37 +1000
То:	svc_t1connectp
Subject:	FW: DA20/0148 - TfNSW Request for Further Information
Attachments:	ptc. Response to TfNSW RFI - 29 06 20.pdf

#ECMBODY

From: Ashleigh Ryan <aryan@urbis.com.au>
Sent: Wednesday, 1 July 2020 9:26 AM
To: Kathryn Saunders <kathryn.saunders@penrith.city>
Cc: Rob Battersby <rbattersby@urbis.com.au>
Subject: RE: DA20/0148 - TfNSW Request for Further Information

EXTERNAL EMAIL: This email was received from outside the organisation. Use caution when clicking any links or opening attachments.

Good Morning Kathy,

I hope you are well.

Further to the email below, please find attached a response to the matters raised by TfNSW from their consideration of the proposed development. Please let me know if you require anything further to forward this onto TfNSW.

All the best, Ashleigh

ASHLEIGH RYAN

ASSOCIATE DIRECTOR

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From: Kathryn Saunders <<u>kathryn.saunders@penrith.city</u>>
Sent: Monday, 1 June 2020 9:05 AM
To: Ashleigh Ryan <<u>aryan@urbis.com.au</u>>
Subject: DA20/0148 - TfNSW Request for Further Information

Good morning Ashleigh,

Please see the attached correspondence from TfNSW with regard to DA20/0148 for your information. If you will be submitting additional information for the consideration of TfNSW in response to the attached, please ensure that this is submitted to Penrith Council initially, so that I may forward on as a formal response.

Regards,

Kathryn Saunders Senior Development Assessment Planner

E <u>kathryn.saunders@penrith.city</u> T <u>+612 4732 8567</u> PO Box 60, PENRITH NSW 2751 <u>www.visitpenrith.com.au</u> www.penrithcity.nsw.gov.au 29 June 2020

Bernardo Reiter Landa Toga Level 5, 45 Jones Street Ultimo NSW 2007

Dear Bernardo

1. DA20/0148 – Response to TfNSW RFI dated 27 May 2020 (Ref. SYD20/00453/01)

This letter has been prepared to present our response to the comments / queries raised by TfNSW relating to the traffic assessment and modelling associated with the subject Development Application.

It should be noted that the modelling accompanies a Development Application, which represents an amendment to the approved development scheme and that the same model formed the basis of the approved development, which was endorsed by Council and RMS.

The modelling was prepared to satisfy the requirements of Council through the preparation of the original development application. In this regard, while we have included the requested changes within the modelling, it is important to highlight that this application is to increase the number of dwellings within the approved development and therefore the fundamental parts of the modelling have been retained as per the previously approved model.

The following section provides a summary of our findings following the incorporation of the TfNSW changes to the model.

2. Executive Summary

Scenario 1A	2020 Existing AM Peak
Scenario 1B	2020 Existing PM Peak
Scenario 2A	2026 Future Base AM Peak (includes Urban Apts)
Scenario 2B	2026 Future Base PM Peak (includes Urban Apts)
Scenario 3A	2026 Future Base plus Development AM Peak
Scenario 3B	2026 Future Base plus Development PM Peak

In summary, in response to the comments provided by TfNSW, we have prepared the following modelling scenarios:

The modelling results are summarised in Attachment 1 and indicate that following the changes recommended by TfNSW, the post development 2026 scenario operates within capacity during the morning and evening peak periods. Key capacity constraints are noted at the intersection of High Street and Worth Street, which currently operates at Level of Service E and continues to do so within the 2026 pre and post-development scenarios.

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The post-development scenario indicates slight changes to some of the key indicators (degree of Saturation and Average Delay) however all of the Levels of Service remain unchanged by the development.

A detailed response to each point raised is presented on the following pages.

The electronic SIDRA files have also been provided.

We trust that this information facilitates the completion of the assessment, however, should any clarification be required, please do not hesitate to contact me.

Your faithfully

Al.

Andrew Morse

Partner

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3. Detailed Response

1	 Generally, this section needs more detail as the following information is not clear: Which intersections use data from TfNSW (including RMS) or Council? Whether existing SCATS data from the signalised intersections was obtained from TfNSW to form the basis of the 2020 design year models? Broadly explain how and why Council had to project data to 2020? What are the adopted peak periods? Which movements have been increased by 10% to represent the 2026 design year? Some of these may have been provided in another document, and they need to be included in this report. As these have not been provided, they could not be reviewed.
Response	The following intersections were based on Council / RMS data:
	High Street / Worth Street = Council - Adopted from Council's Town Centre model
	Worth Street / Union Lane = Council
	Worth Street / Union Road = Council
	Mulgoa Road / Union Road = RMS - Adopted from the RMS Jane Street model
	Mulgoa Road / High Street = RMS
	High Street / Civic Centre = ptc. survey (was not included in the Council or RMS model)
	SCATS data was used in the original version of the modelling in 2017.
	To provide some context, when the original model was established in 2017, we were assessing a development with a new road link, a potential major upgrade to Mulgoa Road, signalisation of the High Street roundabout in the context that Council were also developing a Town Centre model to project growth on the network and the need for upgrades.
	In terms of matching the data, it was agreed with Council that we would adopt the data from the Town Centre strategic model, which was being developed at the time of the original DA. In 2017 we agreed that 2020 would form the base year as it was anticipated that parts of the development would be complete and potentially the RMS Jane Street project. 2026 was agreed as the post development year, given that there are further stages of the Toga project on the western site, which could be completed by 2026. The RMS model was already set up for 2026, therefore the Council data was increased to match the RMS data (the growth was applied equally to all movements except those associated with the Westfield car park at Worth Street.
	The peak periods were established as 8:00-9:00am and 4:00-5:00pm.
2.	It is recommended that more detail about existing conditions such as survey counts and survey dates, queue lengths or any other data such as signal timings etc. used for model development and calibration be included in Section 3 or as an appendix of the report.
Response	The difficulty in providing validation based on existing conditions relates to the changes to the road network that are occurring in the vicinity of the site and the fact that the modelling base year was 3 years in the future. The current DA was prepared during COVID-19 restrictions, so updated traffic surveys have not been possible.
	Given that the current DA is an amendment to an approved DA (and accepted modelling assessment) it was not considered necessary to undertake a new model, but to apply the increased traffic projection to the current modelling.
3.	Generally, pre and post development impacts are compared for the same future year, i.e. how does the network operate in 2026 with and without the development. The approach

	taken (page 13) is different, and it is recommended that the report and modelling consider the following scenarios:
	• 2020 – Existing Conditions: Existing Road Network, no development
	• 2026 – Future Base: Future Road Network, growth, no development
	• 2026 – Future Base plus development and Link Road
Response	This is a valid recommendation and these models have now been prepared to demonstrate the difference between the 2026 with/without development scenarios
4.	Figure 11 and 12 are labelled "Existing Road Network, Post Development". The description on page 13 suggests that it should be "Pre-Development".
Response	Noted, these should have read 'pre-development'
5.	The volume of eastbound and westbound traffic along Union Road in the AM and PM peaks changes by at least 100vph between Worth Street and Mulgoa Road. It is not clear if this is a result of traffic generating developments, or a function of the 2020 volume development process. It is recommended that the volumes are reviewed, update if required and an explanation provided.
Response	We note that there is some loss/gain between intersections as a result of using the Council and RMS traffic volumes on the east and west side of the model. We chose not to make manual adjustments in order to maintain the datasets provided by RMS and Council. The example identified shows a decrease in westbound traffic in the morning, but an increase in the afternoon, therefore a simple increase to make up the difference is not appropriate. Some traffic would have been lost / gained by the former use of the site as a car sales yard and an unofficial car park.
6.	The report identifies that model "calibration" has been applied to the intersection of Mulgoa Road / Union Road and gap acceptance parameters changed for the south approach right turn to ensure that the 95th percentile back of queue length is contained within the existing right turn lane.
	Given that Mulgoa Road is under construction to be upgraded to three lanes in each direction, and the right turn volumes at Mulgoa Road/ Union Road are projected volumes, the calibration will not result in replicating the typical traffic condition.
	Any default parameters should only be changed to replicate existing behaviour and adequate justification should be included in the report.
Response	Observations on site at the time of the modelling indicate that the gap acceptance for the right turn movement is less that the default setting in SIDRA. When running the model we observe queuing that does not occur on site and calibrated the model to reflect the on-site observations (this was prior to the construction work). The change to the default setting is made exactly for this reason, to ensure that the model reflects actual performance.
7.	Section 3.3 discusses future road network amendments, including intersection upgrades to Mulgoa Road and the Mulgoa Road / High Street intersection. These changes have already been included in the model results presented in Section 3.2.3 and therefore should already be described earlier in the report or the models updated to show the existing conditions road network.
Response	At the time of preparing the original model in 2017, the 2020 scenario anticipated that the Mulgoa Road upgrade would be completed. As of June 2020, the works are still underway, therefore we have rerun the 2020 model with the current (not upgraded) road network to reflect the current conditions.

8.	It is recommended that development traffic distribution assumptions be included in the report. To provide a clear understanding of the development impacts, it is prudent to understand how the development traffic was distributed on the network.
Response	The traffic associated with the development was distributed according to the current distribution patterns at each intersection and the directional split on each road. The traffic flow diagrams are presented in Section 3.2 of the Traffic Impact Assessment.
9.	The report mentions that the traffic generated by the Urban Apartments development has been included within the Post-Development modelling scenario. This should be described clearly in the modelling scenarios discussed in Item 3, and could be included at the future base scenario, or as a further post development scenario, depending on which development is likely to be constructed first.
Response	We have included the Urban Apartments development within the 2026 base model (noting that we were requested by Council to include this development within the post-development model, which was accepted in the context of the approved DA).
10.	The report comments that some delays experienced in the network at the intersections will increase from the existing conditions but "the proposal will result in marginal increases to those delays".
	A significant change in operation is expected at the intersections of Mulgoa Road / Union Road, Worth Street / Union Road and High Street / Worth Street, and therefore the comment is currently considered inaccurate. It may be clarified or justified if the build-up of design year scenarios is improved.
Response	The new 2026 'without development' scenario enables a clear comparison of the pre and post development situations.
	Generally, while the development results in some increases to the average delays on those movements where additional traffic is applied, none of the intersections result in a high LoS and none of the intersections have an overall LoS of D or less, other than Worth Street / High Street, which operates at LoS E during the PM peak in both pre and post development scenarios (this intersection operates at LoS E at present).
11.	The report describes the future operation of the Mulgoa Road / Union Road intersection, summarising that the unsignalised right turn exceeds the capacity of the movement in the peak periods. The report comments that this "is likely due to the random arrival of vehicles from the north within the two southbound lanes", yet both the existing and future year models include the three-lane southbound layout.
	This section also mentions that "The operation of the High Street intersection likely creates gaps between the signal phases that are not being replicated in SIDRA".
	This statement is also considered inaccurate as this is one of the main purposes of developing SIDRA Network models. It is noted that providing correct phasing in a logical sequence may improve the anticipated operation of this movement
	It is recommended that this section of the report is reviewed as it is not accurate and revised.
Response	It is noted that the report should have referred to three southbound lanes, but the comment relating to random arrivals is still valid. While the southbound lanes of Mulgoa Road are free flowing, there are little opportunities for the right turn movement, however, we observed on site that the changes to the phases at the High Street intersection produced gaps of sufficient length to enable the lead vehicle and some follow-up vehicles to make the turn. The initial modelling did not reflect this outcome. Hence, we calibrated with the gap acceptance adjustment to result in more realistic queue lengths. The intersection operates similarly when run within the network or in isolation.

12.	The report concludes that : "In summary, the model indicates that the proposal will be accommodated within the road network and will result in some manageable increase to the delays at some intersections"
	As per the results presented in Table 17 and 18 of the report, delays at some intersections increase in the order of 500 seconds and LOS drops from C/D to F with the proposed development. Furthermore, three intersections are anticipated to operate with a DOS over 1. This indicates that the intersections are operating above capacity.
	As highlighted in Item 3, it is recommended that new 2026 future base models are developed with background growth but without the development to understand which aspect of traffic is likely to causes the poor operation. If development traffic causes the poor operation of the intersections, then mitigation options will need to be developed and assessed for the intersections which are operating over capacity.
Response	Noted: The increase in poor results is related to a comparison between the 2020 and 2026 post development scenarios. A comparison between the 2026 base case and 2026 post development scenarios presents a more appropriate basis for the assessment. Many of the poor results noted by TfNSW in the post development model are also identified in the 2026 base case, indicating that the background traffic growth that has been applied to the model causes the performance of the intersections to decrease.
	The proposed revised development results in very little change to the results, for example, all of the overall intersection LoS results are the same in both the pre and post development scenarios (AM and PM). In this regard, there are no mitigation works are required as a result of the development.
	The right turn movement from Mulgoa Road into Union Road operates with a high degree of saturation under both scenarios and this results in a sizable increase in the average delay and queue lengths in the post development scenario, however only a small number of development-generated vehicles are added to this movement. This disproportionate result is the effect of adding traffic to a highly saturated movement.
13.	It does not appear that pedestrian surveys have been undertaken. It is recommended that pedestrian information is collected for the signalised intersections on Worth Street as the intersection operation will be affected by vehicles giving way to pedestrians.
Response	The data for pedestrians was not included within either the Council or RMS dataset, therefore the default values of 50 pedestrians per hour was retained on each crossing. This is sufficient to call up a crossing movement on each cycle and was therefore considered a suitable assumption.
14.	Approach cruise speeds and exit cruise speeds should match the posted speed limits at Union Road, Worth Street and Union Lane.
Response	Noted. This has been checked and changed where necessary.
15.	Gap acceptance parameters have been adjusted from their default values at the intersection of Mulgoa Road / Union Road.
	<i>As discussed in Item 6, this is not considered appropriate given that both the layout and volumes represent future conditions.</i>
Response	See response to Item 6
16.	All lane widths for all intersections are the default 3.3m. Throughout the intersection network, there are lane widths ranging from 2.5m to 3.3m. It is recommended that these are updated.
Response	Noted. This has been checked and changed where necessary.
17.	Intersection geometry is incorrect at the Worth Street / Union Lane intersection as follows:

	East approach – right turn lane should be a short lane with parking
	South approach- should have two exit lanes and two approach lanes.
Response	Noted. This has been checked and changed where necessary.
18.	Intersection geometry is incorrect at the Worth Street / Union Road intersection as follows:
	North approach – kerb side lane should be a left turn only lane, without parking.
	East approach – kerb side lane should be a short lane with parking.
Response	Noted. This has been checked and changed where necessary.
19.	On the north approach at High Street / Worth Street the left turn slip lane has been modelled as a separate lane. SIDRA recommends that slip lanes with a length less than 30m are modelled as a slip lane off the through lane and Free Queue distances added in the Lane Disciplines to inform how queue lengths for through and left turn vehicles interact. It is recommenced that this is updated.
Response	Noted. This has been checked and changed and we note for future reference.
20.	The intersection control needs to be updated at the Worth Street / Union Lane intersection to be a Give-Way rather than a Stop.
Response	Noted. This has been checked and changed.
21.	It is noted that pedestrian protection is not included for any of the pedestrian movements. It is likely pedestrian protection will be included from left turn vehicles and some right turn movements at the signalised intersections on Worth Street. The model should include pedestrian protection for all conflicting movements in line with TfNSW's Traffic Signal Design Manual and specific advice from the TfNSW Network Operations teams. If pedestrian protection is provided at the sites, pedestrian movements must be included within the priorities, and where required the length of late start for vehicles added under Gap Acceptance. Opposing Peds (cignals)
Response	Pedestrian protection was not operating at the modelled intersections at the time of the modelling. It is unknown whether it is operating now or if not when it will be introduced to these intersections. Regardless, it is relevant to apply the same setting to the post and predevelopment scenarios to provide an accurate comparison.
22.	At High Street and Mulgoa Road, the adopted traffic signal phase sequence of A, C, F2 would be unconventional and must be reviewed. Phasing for all signalised intersections requires review and should be set up as per SCATS data. Further detail is required in the report on how the phasing sequence and the phases were identified.
Response	The phasing has been adjusted based on the TCS drawing phase diagram.
23.	At High Street / Worth Street, the adopted phasing should be obtained from SCATS.
	The phasing has been established based on the TCS drawing phase diagram.
24.	Phasing and timing at Worth Street / Union Road needs to be the same as SCATS. Any changes to this phasing also should be consulted with network operations in TfNSW.
	The 2026 model deals with future traffic growth and changes to the intersection geometry, therefore the SCATS data is not applicable to the future scenarios.

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25.	It is recommended that using 'Undetected' movements be considered and phase transitions for related signalised left turn movements be considered and reviewed for the signalised intersections along Worth Street.
	'Undetected movements' has been applied to the slip lane at the intersection of High Street / Worth Street.
26.	All changes recommended above should be undertaken on the 2026 post development models.
	Noted and included
27.	Approach and exit cruise speeds have been kept as the default 60km/h for new roads. It is recommended that this is reviewed to ensure that this is correct.
	Noted. This has been checked and changed where necessary.
28.	The intersection phasing adopted at High Street / Mulgoa Road needs to be completely reviewed as described in the above sections. Furthermore, it is noted that the addition of a user class for the Urban Apartments has affected the phasing for the left turn slip lane from the east approach and there are now major vehicle conflicts. Refer to Figure 1 for an example.
	The phasing issue has been corrected and the Urban Apartments traffic volume has been run within the 2026 base case.

Document Control: Prepared by AM on 29 June 2020. Reviewed by HL on 29 June 2020.

Attachment 1 - Sidra Intersection Summaries

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Site: 1 [1. High St and Mulgoa Rd - No Upgrade]

High Street and Mulgoa Road 2020 Existing Existing Road Network, No Dev Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Movement Performance - Vehicles Arrival Flows <u>Total</u> HV Mov **Demand Flows** Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No.Average Satn Total Delay Service Vehicles Distance Queued Stop Cycles Speed Rate % veh/h % km/h veh/h sec South: Mulgoa Road LOS B 0.69 0.73 0.69 12 140 2.0 140 2.0 0 194 25.2 4.9 34.7 36.4 1 2 T1 1023 2.0 1023 2.0 1.129 194.2 LOS F 34.4 244.8 1.00 1.68 2.04 10.0 3 R2 LOS F 67.3 1.00 1.48 119 2.0 119 2.0 0.919 91.9 9.5 1.00 5.9 Approach 1282 2.0 1282 2.0 1.129 166.3 LOS F 34.4 244.8 0.97 1.51 1.84 10.8 East: High Street 4 L2 59 2.0 58 2.0 0.205 41.0 LOS C 5.8 41.3 0.76 0.75 1.01 10.7 5 T1 175 2.0 171 0.205 LOS C 413 0 79 0.69 0.89 29.2 20 39.5 5.8 R2 6 105 2.0 103 2.0 0.376 64.1 LOS E 6.4 45.8 0.95 0.78 0.95 22.1 Approach 339 2.0 332^N 2.0 0.376 47.4 LOS D 6.4 45.8 0.84 0.73 0.93 24.5 North: Castlereagh Road 100.4 7 12 148 148 2.0 0.991 LOS F 592 421.2 1.00 1.20 1.42 14 7 2.0 T1 8 1058 2.0 1058 2.0 0.991 94.5 LOS F 59.2 421.2 1.00 1.22 1.43 14.9 9 R2 498 2.0 498 2.0 1.126 172.3 LOS F 25.4 180.8 1.00 1.30 2.11 13.5 1704 Approach 2.0 1704 2.0 1.126 117.7 LOS F 59.2 421.2 1.00 1.24 1.63 14.2 West: High Street 10 12 895 2.0 895 20 0.562 31.0 LOSIC 16 5 117.4 0.80 0.88 0.97 39.5 T1 501 2.0 501 2.0 1.129 LOS F 66.1 470.9 1.00 1.68 2.04 8.1 11 194 7 12 R2 294 294 2.0 1.081 LOS F 33.9 241.3 1.00 1.26 1.91 9.3 2.0 166 2 Approach 1689 2.0 1689 2.0 1.129 103.1 LOS F 66.1 470.9 0.89 1.18 1.45 18.5 2.0 **5008**^{N1} 5015 2.0 LOS F 66.1 470.9 0.94 All Vehicles 1.129 120.5 1.26 1.58 14.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	48.2	LOS E	0.2	0.2	0.83	0.83				
P1S	Crossing	53	30.2	LOS D	0.1	0.1	0.90	0.90				
P2	East Full Crossing	53	41.7	LOS E	0.2	0.2	0.77	0.77				

P3 P4S	North Full Crossing West Slip/Bypass Lane	53 53	50.7 43.3	LOS E	0.2 0.2	0.2 0.2	0.85 0.79	0.85 0.79
1.10	Crossing		10.0	200 2	0.2	0.2	0.70	0.10
All Peo	destrians	263	42.8	LOS E			0.83	0.83

abla Site: 2 [2. Mulgoa Rd and Union Rd - No Upgrade]

♦♦ Network: N101 [Network Model - 2020 Existing AM Peak]

Mulgoa Rd and Union Rd 2020 Existing Existing Road Network, No Dev Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Ver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Mulgo	ba Road												
2	T1	1309	2.0	1309	2.0	0.498	2.4	LOS A	34.6	246.4	0.21	0.00	0.27	55.7
3	R2	302	2.0	302	2.0	1.217	236.6	LOS F	42.3	301.4	1.00	3.51	10.49	6.8
Appro	ach	1612	2.0	1612	2.0	1.217	46.3	NA	42.3	301.4	0.36	0.66	2.19	23.7
East:	Union	Road												
4	L2	60	2.0	60	2.0	0.074	8.4	LOS A	0.3	2.1	0.45	0.66	0.45	49.7
Appro	ach	60	2.0	60	2.0	0.074	8.4	LOS A	0.3	2.1	0.45	0.66	0.45	49.7
North	: Mulgo	a Road												
7	L2	334	2.0	329	2.0	0.179	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	38.5
8	T1	1084	2.0	1067	2.0	0.277	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	1418	2.0	<mark>1396</mark> ^{N²}	2.0	0.277	1.3	NA	0.0	0.0	0.00	0.14	0.00	57.6
All Ve	hicles	3089	2.0	<mark>3067</mark> ^{N*}	¹ 2.0	1.217	25.1	NA	42.3	301.4	0.20	0.42	1.16	33.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 3 [3. High St and Civic Roundabout]

♦♦ Network: N101 [Network Model - 2020 Existing AM Peak]

High and Civic Roundabout 2020 Existing Existing Road Network, No Dev Site Category: (None) Roundabout

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand F Total	Flows HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Nate		km/h
East:	High S	t (E)												
5	T1	313	2.0	310	2.0	0.112	2.1	LOS A	0.5	3.2	0.08	0.27	0.08	41.8
6	R2	28	2.0	28	2.0	0.112	7.5	LOS A	0.4	3.2	0.08	0.32	0.08	50.6
Appro	bach	341	2.0	339 ^{N1}	2.0	0.112	2.6	LOS A	0.5	3.2	0.08	0.27	0.08	43.6
North	: Civic I	PI (N)												
7	L2	17	2.0	17	2.0	0.042	4.0	LOS A	0.2	1.4	0.45	0.57	0.45	42.2
9	R2	26	2.0	26	2.0	0.042	9.2	LOS A	0.2	1.4	0.45	0.57	0.45	42.2
Appro	bach	43	2.0	43	2.0	0.042	7.1	LOS A	0.2	1.4	0.45	0.57	0.45	42.2
West	High S	St (W)												
10	L2	74	2.0	68	2.0	0.230	2.6	LOS A	1.1	7.9	0.07	0.25	0.07	48.1
11	T1	695	2.0	643	2.0	0.230	2.1	LOS A	1.1	7.9	0.07	0.24	0.07	41.0
Appro	bach	768	2.0	<mark>711</mark> N1	2.0	0.230	2.2	LOS A	1.1	7.9	0.07	0.25	0.07	42.9
All Ve	hicles	1153	2.0	1093 ^{N1}	2.1	0.230	2.5	LOS A	1.1	7.9	0.09	0.27	0.09	43.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 4 [4. High St and Worth St]

High and Worth 2020 Existing Existing Road Network, No Dev Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Mov	rement Performance - Vehicles Turn Demand Flows Arrival Flows Deg Average Level of 95% Back of Queue Prop Effective Aver No Average													
Mov	Turn	Demand I	Flows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective /	Aver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Nate		km/h
South	n: Worth	n St (S)												
1	L2	344	2.0	340	2.0	0.506	29.6	LOS C	13.8	97.9	0.75	0.78	0.75	7.8
2	T1	127	2.0	126	2.0	0.286	21.6	LOS B	7.8	55.3	0.66	0.64	0.66	32.1
3	R2	91	2.0	89	2.0	0.286	25.9	LOS B	7.8	55.3	0.66	0.64	0.66	31.8
Appro	oach	562	2.0	<mark>554</mark> N1	2.0	0.506	27.2	LOS B	13.8	97.9	0.71	0.72	0.71	20.9
East:	High S	t (E)												
4	L2	28	2.0	28	2.0	0.240	34.8	LOS C	6.4	45.4	0.76	0.65	0.76	27.2
5	T1	282	2.0	282	2.0	0.240	30.2	LOS C	6.5	46.6	0.76	0.64	0.76	27.4
6	R2	31	2.0	31	2.0	0.110	40.9	LOS C	1.4	9.8	0.79	0.71	0.79	31.8
Appro	oach	341	2.0	341	2.0	0.240	31.6	LOS C	6.5	46.6	0.76	0.64	0.76	28.0
North	: Worth	St (N)												
7	L2	1	2.0	1	2.0	0.014	17.9	LOS B	0.4	2.9	0.47	0.34	0.47	42.2
8	T1	14	2.0	14	2.0	0.014	13.4	LOS A	0.4	2.9	0.47	0.34	0.47	36.6
9	R2	16	2.0	16	2.0	0.041	20.7	LOS B	0.4	3.1	0.67	0.64	0.67	32.2
Appro	oach	31	2.0	31	2.0	0.041	17.3	LOS B	0.4	3.1	0.57	0.49	0.57	34.5
West	: High S	St (W)												
10	L2	118	2.0	110	2.0	0.209	36.0	LOS C	4.6	33.1	0.76	0.74	0.76	28.9
11	T1	451	2.0	420	2.0	0.321	31.2	LOS C	9.0	64.3	0.78	0.66	0.78	30.5
12	R2	199	2.0	186	2.0	0.569	45.0	LOS D	9.5	67.6	0.91	0.82	0.91	10.9
Appro	oach	767	2.0	<mark>716</mark> ^{N1}	2.0	0.569	35.5	LOS C	9.5	67.6	0.81	0.71	0.81	26.3
All Ve	ehicles	1701	2.0	1642 ^{N1}	2.1	0.569	31.6	LOS C	13.8	97.9	0.76	0.70	0.76	25.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		
P1	South Full Crossing	53	32.3	LOS D	0.1	0.1	0.73	0.73
P2	East Full Crossing	53	18.7	LOS B	0.1	0.1	0.56	0.56
P3	North Full Crossing	53	33.8	LOS D	0.1	0.1	0.75	0.75

P3S	North Slip/Bypass Lane Crossing	53	27.4	LOS C	0.1	0.1	0.68	0.68
P4 P4S	West Full Crossing West Slip/Bypass Lane Crossing	53 53	26.1 54.3	LOS C LOS E	0.1 0.2	0.1 0.2	0.66 0.95	0.66 0.95
All Peo	destrians	316	32.1	LOS D			0.72	0.72

Site: 5 [5. Worth St and Union Ln]

♦♦ Network: N101 [Network Model - 2020 Existing AM Peak]

Worth St and Union Ln 2020 Existing Existing Road Network, No Dev Site Category: (None) Stop (Two-Way)

Move	vement Performance - Vehicles 7 Turn Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No.Average													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rale		km/h
South	: Worth	n St (S)												
1	L2	74	2.0	72	2.0	0.040	3.9	LOS A	0.0	0.0	0.00	0.52	0.00	29.7
2	T1	442	2.0	434	2.0	0.226	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
Appro	ach	516	2.0	<mark>507^{N1}</mark>	2.0	0.226	0.6	NA	0.0	0.0	0.00	0.07	0.00	42.8
East:	Union	Ln (E)												
4	L2	18	2.0	18	2.0	0.059	3.9	LOS A	0.2	1.7	0.34	0.50	0.34	35.0
5	T1	20	2.0	20	2.0	0.059	9.1	LOS A	0.2	1.7	0.34	0.50	0.34	35.1
6	R2	25	2.0	25	2.0	0.130	10.9	LOS A	0.2	1.7	0.61	0.80	0.61	32.3
Appro	ach	63	2.0	63	2.0	0.130	8.4	LOS A	0.2	1.7	0.45	0.62	0.45	34.0
North:	Worth	n St (N)												
8	T1	143	2.0	135	2.0	0.071	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
9	R2	97	2.0	91	2.0	0.103	6.8	LOS A	0.4	2.6	0.41	0.64	0.41	25.2
Appro	ach	240	2.0	<mark>227</mark> N1	2.0	0.103	2.7	NA	0.4	2.6	0.17	0.26	0.17	32.6
West:	Union	Ln (W)												
10	L2	36	2.0	36	2.0	0.081	6.0	LOS A	0.3	2.2	0.52	0.70	0.52	20.0
12	R2	15	2.0	15	2.0	0.081	12.2	LOS A	0.3	2.2	0.52	0.70	0.52	20.0
Appro	ach	51	2.0	51	2.0	0.081	7.8	LOS A	0.3	2.2	0.52	0.70	0.52	20.0
All Ve	hicles	869	2.0	<mark>847</mark> N1	2.1	0.226	2.1	NA	0.4	2.6	0.11	0.20	0.11	35.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 6 [6. Worth St and Union Rd]

Worth St and Union Rd 2020 Existing Existing Road Network, No Dev Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 130 seconds (Site User-Given Cycle Time)

Move	wement Performance - Vehicles													
Mov	Turn	Demand I	Flows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	Aver. No.A	verage
U		Iotai	ΗV	Iotai	ΗV	Sath	Delay	Service	venicies	Distance	Queued	Stop Rate	Cycles 5	peed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Worth	n St (S)												
1	L2	22	2.0	22	2.0	0.390	68.8	LOS E	3.5	24.6	0.99	0.75	0.99	17.8
2	T1	20	2.0	20	2.0	0.390	64.2	LOS E	3.5	24.6	0.99	0.75	0.99	17.8
3	R2	13	2.0	13	2.0	0.390	68.8	LOS E	3.5	24.6	0.99	0.75	0.99	25.8
Appro	bach	55	2.0	55	2.0	0.390	67.2	LOS E	3.5	24.6	0.99	0.75	0.99	20.1
East:	Union	Rd (E)												
4	L2	4	2.0	4	2.0	0.129	13.1	LOS A	3.6	26.0	0.39	0.34	0.39	44.3
5	T1	156	2.0	156	2.0	0.129	8.5	LOS A	3.6	26.0	0.39	0.34	0.39	40.6
6	R2	398	2.0	398	2.0	0.678	21.3	LOS B	15.7	111.7	0.67	0.78	0.67	31.7
Appro	bach	558	2.0	558	2.0	0.678	17.7	LOS B	15.7	111.7	0.59	0.65	0.59	33.9
North	: Worth	St (N)												
7	L2	109	2.0	105	2.0	0.615	67.8	LOS E	6.7	47.5	1.00	0.80	1.03	18.6
8	T1	7	2.0	7	2.0	0.061	55.7	LOS D	0.7	5.3	0.92	0.66	0.92	20.7
9	R2	6	2.0	6	2.0	0.061	59.6	LOS E	0.7	5.3	0.92	0.66	0.92	3.7
Appro	bach	123	2.0	<mark>118</mark> ^{N1}	2.0	0.615	66.6	LOS E	6.7	47.5	0.99	0.78	1.02	18.3
West	: Union	Rd (W)												
10	L2	96	2.0	87	2.0	0.069	8.6	LOS A	1.4	10.2	0.27	0.58	0.27	36.6
11	T1	266	2.0	242	2.0	0.211	8.9	LOS A	5.8	41.2	0.41	0.37	0.41	43.3
12	R2	5	2.0	5	2.0	0.211	13.6	LOS A	5.8	41.2	0.42	0.36	0.42	42.5
Appro	bach	367	2.0	<mark>333</mark> N1	2.0	0.211	8.9	LOS A	5.8	41.2	0.37	0.42	0.37	42.4
All Ve	hicles	1103	2.0	1063 ^{N1}	2.1	0.678	22.9	LOS B	15.7	111.7	0.59	0.60	0.59	32.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	9.3	LOS A	0.1	0.1	0.38	0.38
P2	East Full Crossing	53	58.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	11.2	LOS B	0.1	0.1	0.42	0.42

P4	West Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96
All Peo	lestrians	211	34.5	LOS D			0.67	0.67

Site: 1 [1. High St and Mulgoa Rd - No Upgrade]

High Street and Mulgoa Road 2020 Existing Existing Road Network, No Dev Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Move	vement Performance - Vehicles													
Mov	Turn	Demand	Flows	Arriva	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rale		km/h
South	: Mulg	oa Road												
1	L2	238	2.0	238	2.0	0.317	23.0	LOS B	7.6	54.2	0.72	0.76	0.72	37.7
2	T1	825	2.0	825	2.0	0.902	69.0	LOS E	32.2	229.4	1.00	1.05	1.23	22.1
3	R2	62	2.0	62	2.0	0.400	73.2	LOS F	4.2	29.7	0.99	0.76	0.99	7.3
Appro	ach	1125	2.0	1125	2.0	0.902	59.5	LOS E	32.2	229.4	0.94	0.97	1.11	23.6
East:	High S	treet												
4	L2	96	2.0	96	2.0	0.923	88.7	LOS F	26.4	187.7	1.00	1.17	1.81	5.4
5	T1	609	2.0	609	2.0	0.923	80.2	LOS F	26.4	187.7	1.00	1.12	1.54	19.3
6	R2	285	2.0	285	2.0	0.948	93.4	LOS F	24.0	170.8	1.00	1.04	1.42	17.3
Appro	ach	991	2.0	991	2.0	0.948	84.8	LOS F	26.4	187.7	1.00	1.10	1.53	17.6
North	: Castle	ereagh Roa	d											
7	L2	106	2.0	106	2.0	0.960	80.4	LOS F	60.7	432.5	1.00	1.12	1.29	17.6
8	T1	1213	2.0	1213	2.0	0.960	74.8	LOS F	60.7	432.5	0.99	1.13	1.29	17.6
9	R2	633	2.0	633	2.0	0.935	64.6	LOS E	17.5	124.4	1.00	1.04	1.44	28.9
Appro	ach	1952	2.0	1952	2.0	0.960	71.8	LOS F	60.7	432.5	0.99	1.10	1.34	21.7
West:	High S	Street												
10	L2	584	2.0	584	2.0	0.380	21.3	LOS B	8.2	58.2	0.73	0.77	0.73	44.0
11	T1	249	2.0	249	2.0	0.679	57.0	LOS E	16.0	113.7	0.99	0.83	0.99	21.3
12	R2	157	2.0	157	2.0	0.527	64.1	LOS E	9.9	70.8	0.97	0.81	0.97	19.7
Appro	ach	991	2.0	991	2.0	0.679	37.1	LOS C	16.0	113.7	0.83	0.79	0.83	34.0
All Ve	hicles	5058	2.0	5058	2.0	0.960	64.8	LOS E	60.7	432.5	0.95	1.01	1.23	23.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	57.7	LOS E	0.2	0.2	0.91	0.91						
P1S	South Slip/Bypass Lane Crossing	53	25.8	LOS C	0.1	0.1	0.84	0.84						
P2	East Full Crossing	53	36.5	LOS D	0.2	0.2	0.72	0.72						

P3	North Full Crossing	53	60.5	LOS F	0.2	0.2	0.93	0.93
P4S	West Slip/Bypass Lane Crossing	53	43.3	LOS E	0.2	0.2	0.79	0.79
All Pedestrians		263	44.8	LOS E			0.84	0.84

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abla Site: 2 [2. Mulgoa Rd and Union Rd - No Upgrade]

♦♦ Network: N101 [Network Model - 2020 Existing PM Peak]

Mulgoa Rd and Union Rd 2020 Existing Existing Road Network, No Dev Site Category: (None) Giveway / Yield (Two-Way)

Move	ovement Performance - Vehicles													
Mov ID	Turn	Demand F Total	lows= HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Mulgo	ba Road												
2	T1	1121	2.0	1121	2.0	0.503	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
3	R2	163	2.0	163	2.0	0.776	43.1	LOS D	4.6	32.7	0.95	1.27	2.10	25.0
Appro	ach	1284	2.0	1284	2.0	0.776	5.6	NA	4.6	32.7	0.12	0.16	0.27	50.7
East:	Union	Road												
4	L2	258	2.0	258	2.0	0.380	11.4	LOS A	2.1	15.0	0.60	0.89	0.77	47.2
Appro	ach	258	2.0	258	2.0	0.380	11.4	LOS A	2.1	15.0	0.60	0.89	0.77	47.2
North	Mulgo	oa Road												
7	L2	109	2.0	109	2.0	0.060	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	38.5
8	T1	1359	2.0	1359	2.0	0.353	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	1468	2.0	1468	2.0	0.353	0.4	NA	0.0	0.0	0.00	0.04	0.00	59.3
All Ve	hicles	3011	2.0	3011	2.0	0.776	3.6	NA	4.6	32.7	0.10	0.17	0.18	54.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 3 [3. High St and Civic Roundabout]

♦♦ Network: N101 [Network Model - 2020 Existing PM Peak]

High and Civic Roundabout 2020 Existing Existing Road Network, No Dev Site Category: (None) Roundabout

Move	vement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective / Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	High S	t (E)												
5	T1	891	2.0	891	2.0	0.637	2.4	LOS A	1.6	11.7	0.20	0.29	0.20	40.7
6	R2	27	2.0	27	2.0	0.637	7.9	LOS A	1.6	11.7	0.20	0.31	0.20	50.4
Appro	bach	918	2.0	918	2.0	0.637	2.6	LOS A	1.6	11.7	0.20	0.29	0.20	41.4
North	: Civic I	PI (N)												
7	L2	52	2.0	52	2.0	0.228	3.4	LOS A	0.7	4.7	0.37	0.58	0.37	42.3
9	R2	100	2.0	100	2.0	0.228	8.6	LOS A	0.7	4.7	0.37	0.58	0.37	42.3
Appro	bach	152	2.0	152	2.0	0.228	6.8	LOS A	0.7	4.7	0.37	0.58	0.37	42.3
West:	High S	St (W)												
10	L2	58	2.0	58	2.0	0.137	2.6	LOS A	0.6	4.4	0.07	0.26	0.07	48.1
11	T1	360	2.0	360	2.0	0.137	2.1	LOS A	0.6	4.4	0.07	0.25	0.07	41.0
Appro	bach	418	2.0	418	2.0	0.137	2.2	LOS A	0.6	4.4	0.07	0.25	0.07	43.5
All Ve	hicles	1487	2.0	1487	2.0	0.637	2.9	LOS A	1.6	11.7	0.18	0.31	0.18	42.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 4 [4. High St and Worth St]

High and Worth 2020 Existing Existing Road Network, No Dev Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Move	ement	Performa	nce -	Vehic	les									
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective /	Aver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rale		km/h
South	n: Worth	n St (S)												
1	L2	198	2.0	198	2.0	0.659	56.1	LOS D	11.2	79.4	0.99	0.83	1.01	4.4
2	T1	111	2.0	111	2.0	0.429	46.8	LOS D	7.3	51.9	0.93	0.76	0.93	23.4
3	R2	28	2.0	28	2.0	0.429	51.0	LOS D	7.3	51.9	0.93	0.76	0.93	23.2
Appro	bach	337	2.0	337	2.0	0.659	52.6	LOS D	11.2	79.4	0.96	0.80	0.97	14.0
East:	High S	t (E)												
4	L2	77	2.0	77	2.0	0.469	33.1	LOS C	13.3	94.8	0.79	0.71	0.79	27.8
5	T1	592	2.0	592	2.0	0.469	28.4	LOS B	15.4	109.7	0.79	0.70	0.79	28.1
6	R2	181	2.0	181	2.0	0.417	35.5	LOS C	8.0	57.0	0.79	0.78	0.79	33.4
Appro	bach	849	2.0	849	2.0	0.469	30.3	LOS C	15.4	109.7	0.79	0.72	0.79	29.7
North	: Worth	St (N)												
7	L2	1	2.0	1	2.0	0.298	23.8	LOS B	6.5	46.2	0.63	0.52	0.63	39.6
8	T1	184	2.0	184	2.0	0.298	19.3	LOS B	6.5	46.2	0.63	0.52	0.63	33.0
9	R2	309	2.0	309	2.0	0.538	26.9	LOS B	11.0	78.3	0.87	0.81	0.87	29.1
Appro	bach	495	2.0	495	2.0	0.538	24.0	LOS B	11.0	78.3	0.78	0.70	0.78	30.5
West	: High S	St (W)												
10	L2	203	2.0	203	2.0	0.481	42.6	LOS D	9.9	70.5	0.87	0.80	0.87	26.9
11	T1	232	2.0	232	2.0	0.151	24.4	LOS B	4.3	30.4	0.67	0.55	0.67	33.3
12	R2	212	2.0	212	2.0	0.955	93.9	LOS F	17.7	126.2	1.00	1.15	1.62	5.9
Appro	bach	646	2.0	646	2.0	0.955	52.8	LOS D	17.7	126.2	0.84	0.82	1.05	20.6
All Ve	hicles	2327	2.0	2327	2.0	0.955	38.5	LOS C	17.7	126.2	0.83	0.76	0.89	24.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestr	ians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	27.4	LOS C	0.1	0.1	0.68	0.68
P2	East Full Crossing	53	22.9	LOS C	0.1	0.1	0.62	0.62
P3	North Full Crossing	53	28.8	LOS C	0.1	0.1	0.69	0.69
P3S	North Slip/Bypass Lane	53	22.9	LOS C	0.1	0.1	0.62	0.62

	Crossing							
P4	West Full Crossing	53	51.5	LOS E	0.2	0.2	0.93	0.93
P4S	West Slip/Bypass Lane	53	34.6	LOS D	0.1	0.1	0.76	0.76
	Crossing							
All Pe	destrians	316	31.3	LOS D			0.72	0.72

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Site: 5 [5. Worth St and Union Ln]

♦ Network: N101 [Network Model - 2020 Existing PM Peak]

Worth St and Union Ln 2020 Existing Existing Road Network, No Dev Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehic	es									
Mov	Turn	Demand F	lows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rale		km/h
South	: Worth	n St (S)												
1	L2	58	2.0	58	2.0	0.032	3.9	LOS A	0.0	0.0	0.00	0.52	0.00	29.7
2	T1	311	2.0	311	2.0	0.161	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
Appro	ach	368	2.0	368	2.0	0.161	0.6	NA	0.0	0.0	0.00	0.08	0.00	42.3
East:	Union	Ln (E)												
4	L2	18	2.0	18	2.0	0.092	4.5	LOS A	0.3	1.8	0.48	0.59	0.48	34.7
5	T1	20	2.0	20	2.0	0.092	9.4	LOS A	0.3	1.8	0.48	0.59	0.48	34.8
6	R2	25	2.0	25	2.0	0.094	11.0	LOS A	0.2	1.7	0.61	0.79	0.61	32.3
Appro	ach	63	2.0	63	2.0	0.094	8.7	LOS A	0.3	1.8	0.53	0.67	0.53	33.8
North:	: Worth	n St (N)												
8	T1	382	2.0	382	2.0	0.141	0.4	LOS A	11.6	82.4	0.08	0.08	0.08	41.9
9	R2	89	2.0	89	2.0	0.141	6.0	LOS A	0.7	4.8	0.28	0.26	0.28	24.7
Appro	ach	472	2.0	472	2.0	0.141	1.4	NA	11.6	82.4	0.12	0.11	0.12	34.9
West:	Union	Ln (W)												
10	L2	1	2.0	1	2.0	0.058	5.2	LOS A	0.2	1.5	0.63	0.80	0.63	14.3
12	R2	20	2.0	20	2.0	0.058	12.5	LOS A	0.2	1.5	0.63	0.80	0.63	14.3
Appro	ach	21	2.0	21	2.0	0.058	12.1	LOS A	0.2	1.5	0.63	0.80	0.63	14.3
All Ve	hicles	924	2.0	924	2.0	0.161	1.8	NA	11.6	82.4	0.11	0.15	0.11	35.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 6 [6. Worth St and Union Rd]

Worth St and Union Rd 2020 Existing Existing Road Network, No Dev Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 130 seconds (Site User-Given Cycle Time)

Move	ement	Performa	ince -	Vehic	les									
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective /	Aver. No.A	verage
U		Iotal	ΗV	Iotal	ΗV	Sath	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Worth	n St (S)												
1	L2	16	2.0	16	2.0	0.333	68.3	LOS E	3.0	21.6	0.98	0.74	0.98	18.0
2	T1	28	2.0	28	2.0	0.333	63.7	LOS E	3.0	21.6	0.98	0.74	0.98	18.0
3	R2	4	2.0	4	2.0	0.333	68.3	LOS E	3.0	21.6	0.98	0.74	0.98	26.1
Appro	bach	48	2.0	48	2.0	0.333	65.6	LOS E	3.0	21.6	0.98	0.74	0.98	19.0
East:	Union	Rd (E)												
4	L2	33	2.0	33	2.0	0.183	25.1	LOS B	5.8	41.0	0.61	0.55	0.61	38.4
5	T1	129	2.0	129	2.0	0.183	20.5	LOS B	5.8	41.0	0.61	0.55	0.61	31.7
6	R2	315	2.0	315	2.0	0.914	72.2	LOS F	24.1	171.7	0.95	1.03	1.32	17.0
Appro	bach	477	2.0	477	2.0	0.914	54.9	LOS D	24.1	171.7	0.83	0.87	1.08	20.9
North	: Worth	St (N)												
7	L2	395	2.0	395	2.0	0.774	52.5	LOS D	11.5	81.6	0.98	0.89	1.03	21.7
8	T1	32	2.0	32	2.0	0.063	34.4	LOS C	1.6	11.4	0.74	0.58	0.74	26.9
9	R2	4	2.0	4	2.0	0.063	38.3	LOS C	1.6	11.4	0.74	0.58	0.74	5.9
Appro	bach	431	2.0	431	2.0	0.774	51.0	LOS D	11.5	81.6	0.96	0.86	1.01	21.9
West	Union	Rd (W)												
10	L2	131	2.0	131	2.0	0.134	16.0	LOS B	3.8	26.9	0.45	0.64	0.45	29.7
11	T1	277	2.0	277	2.0	0.411	21.7	LOS B	10.6	75.2	0.65	0.57	0.65	36.4
12	R2	11	2.0	11	2.0	0.411	26.8	LOS B	10.6	75.2	0.66	0.57	0.66	35.7
Appro	bach	418	2.0	418	2.0	0.411	20.0	LOS B	10.6	75.2	0.59	0.59	0.59	35.1
All Ve	hicles	1374	2.0	1374	2.0	0.914	43.5	LOS D	24.1	171.7	0.81	0.78	0.90	24.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedest	rians						
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
U	Description	ped/h	Delay sec	Service	Pedestrian ped	Distance	Queued	Stop Rate
P1	South Full Crossing	53	20.5	LOS C	0.1	0.1	0.56	0.56
P2	East Full Crossing	53	37.8	LOS D	0.1	0.1	0.76	0.76
P3	North Full Crossing	53	23.5	LOS C	0.1	0.1	0.60	0.60
P4	West Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96

All Pedestrians 211 35.3 LOS D 0.72 0.72
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Site: 1 [1. High St and Mulgoa Rd]

♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

High Street and Mulgoa Road 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Move	ement	Performa	nce -	Vehicle	s									
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	vic	202		veh	m		Rate		km/h
South	n: Mulgo	ba Road (S))	VCH/H	/0	V/C	300		VCIT		_			NI11/11
1	L2	158	2.0	158	2.0	0.224	23.4	LOS B	5.0	35.2	0.71	0.74	0.71	37.5
2	T1	1131	2.0	1131	2.0	0.707	49.4	LOS D	23.2	165.0	0.96	0.83	0.96	27.2
3	R2	105	2.0	105	2.0	0.473	68.8	LOS E	6.9	48.9	0.98	0.79	0.98	8.1
Appro	bach	1394	2.0	1394	2.0	0.707	47.9	LOS D	23.2	165.0	0.94	0.82	0.94	26.7
Fact	High S	troot (E)												
	12	ueer (⊏) 33	3.2	32	33	0.053	38.8	1050	15	10.5	0.71	0.60	0.71	10.6
5	L2 T1	102	17	101	17	0.000	50.0		5.4	20.0	0.71	0.03	0.71	25.0
5	11	103	1.7	101	1.7	0.271	54.5 69.0		2.4	30.3 27.1	0.91	0.72	0.91	20.0
0 Annre	RZ	121	1.7	120	1.0	0.205	57.0		5.0	27.1	0.90	0.75	0.90	21.7
Appro	Dach	337	1.9	<mark>333</mark>	1.9	0.265	57.9	L05 E	5.4	30.3	0.91	0.73	0.91	22.9
North	: Castle	ereagh Roa	d (N)											
7	L2	278	2.0	278	2.0	0.220	10.2	LOS A	5.0	35.4	0.36	0.66	0.36	45.4
8	T1	1200	2.0	1200	2.0	0.483	31.2	LOS C	19.6	139.2	0.78	0.68	0.78	30.3
9	R2	593	2.0	593	2.0	0.708	33.7	LOS C	10.4	74.1	0.98	0.84	0.98	38.7
Appro	bach	2071	2.0	2071	2.0	0.708	29.1	LOS C	19.6	139.2	0.78	0.72	0.78	34.8
West	High S	Street (\M/)												
10	12	887	2.0	887	2.0	0 547	23.7	LOSB	14 7	104.8	0 79	0.81	0.81	42.8
11	T1	478	2.0	478	2.0	0.717	61.0		15.8	112.5	1 00	0.86	1 03	20.5
12	P2	203	2.0	203	2.0	0.600	72.0		10.0	71.6	1.00	0.00	1.03	18.1
Appre	nz	1659	2.0	1659	2.0	0.099	12.9		10.1	112.5	0.00	0.04	0.02	21.2
Appro	Jauri	1000	2.0	1000	2.0	0.717	43.4	L05 D	15.0	112.3	0.00	0.03	0.92	31.Z
All Ve	hicles	5459	2.0	<mark>5455</mark> N1	2.0	0.717	40.0	LOS C	23.2	165.0	0.86	0.78	0.87	30.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		
P11	South Stage 1	53	56.8	LOS E	0.2	0.2	0.90	0.90

P12	South Stage 2 South Slip/Bypass Lane	53	61.4	LOS F	0.2	0.2	0.94	0.94
P15	Crossing	53	12.9	LOS B	0.1	0.1	0.59	0.59
P2	East Full Crossing	53	35.1	LOS D	0.1	0.1	0.71	0.71
P2S	East Slip/Bypass Lane	53	13.3	LOS B	0.1	0.1	0.44	0.44
	Crossing							
P31	North Stage 1	53	59.6	LOS E	0.2	0.2	0.92	0.92
P32	North Stage 2	53	26.8	LOS C	0.1	0.1	0.87	0.87
P3S	North Slip/Bypass Lane	53	0.5	LOS A	0.0	0.0	0.11	0.11
	Crossing							
P4	West Full Crossing	53	51.5	LOS E	0.2	0.2	0.86	0.86
P4S	West Slip/Bypass Lane	53	17.6	LOS B	0.1	0.1	0.67	0.67
	Crossing							
All Pe	destrians	526	33.6	LOS D			0.70	0.70

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

V Site: 2 [2. Mulgoa Rd and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

Mulgoa Rd and Union Rd 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce -	Vehicl	es									
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Mulgo	ba Road (S))											
2	T1	1399	2.0	1399	2.0	0.651	5.4	LOS A	6.8	48.4	0.23	0.00	0.38	51.0
3	R2	368	2.0	368	2.0	1.532	508.4	LOS F	87.7	624.3	1.00	5.27	17.43	3.4
Appro	ach	1767	2.0	1767	2.0	1.532	110.3	NA	87.7	624.3	0.39	1.10	3.93	12.9
East:	Union	Road (E)												
4	L2	204	2.6	204	2.6	0.228	6.2	LOS A	0.9	6.5	0.41	0.64	0.41	47.6
Appro	ach	204	2.6	204	2.6	0.228	6.2	LOS A	0.9	6.5	0.41	0.64	0.41	47.6
North:	Mulgo	a Road (N))											
7	L2	327	1.9	327	1.9	0.179	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	38.5
8	T1	1245	2.0	1245	2.0	0.216	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
Appro	ach	1573	2.0	<mark>1572</mark> ^N	¹ 2.0	0.216	1.2	NA	0.0	0.0	0.00	0.12	0.00	58.0
All Ve	hicles	3544	2.0	3544	2.0	1.532	55.9	NA	87.7	624.3	0.22	0.64	1.98	21.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

V Site: 3 [3. High St and Civic Roundabout]

♦♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

High and Civic Roundabout 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Roundabout

Move	ement	Performa	nce -	Vehicle	es									
Mov ID	Turn	Demand I Total	Flows HV	Arrival F Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	High S	t (E)												
5	T1	318	0.0	314	0.0	0.111	2.1	LOS A	0.5	3.2	0.07	0.27	0.07	42.0
6	R2	31	0.0	30	0.0	0.111	7.5	LOS A	0.5	3.2	0.07	0.32	0.07	50.6
Appro	bach	348	0.0	<mark>344</mark> N1	0.0	0.111	2.5	LOS A	0.5	3.2	0.07	0.28	0.07	43.8
North	: Civic	PI (N)												
7	L2	12	0.0	12	0.0	0.033	4.2	LOS A	0.1	1.0	0.49	0.61	0.49	41.9
9	R2	21	0.0	21	0.0	0.033	9.4	LOS A	0.1	1.0	0.49	0.61	0.49	41.9
Appro	bach	33	0.0	33	0.0	0.033	7.6	LOS A	0.1	1.0	0.49	0.61	0.49	41.9
West	High S	St (W)												
10	L2	74	0.0	74	0.0	0.275	2.6	LOS A	1.4	9.9	0.08	0.25	0.08	48.0
11	T1	787	0.0	787	0.0	0.275	2.1	LOS A	1.4	9.9	0.09	0.25	0.09	40.7
Appro	bach	861	0.0	861	0.0	0.275	2.2	LOS A	1.4	9.9	0.09	0.25	0.09	42.5
All Ve	hicles	1242	0.0	1237 ^{N1}	0.0	0.275	2.4	LOS A	1.4	9.9	0.09	0.26	0.09	42.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Site: 4 [4. High St and Worth St]

♦♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

High and Worth 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Movement Performance - Vehicles														
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		voh/h	0/_	voh/h	0/_	vic	200		yoh	m		Rate		km/h
South: Worth St (S)		/0	VEII/II	70	V/C	360	_	Ven	111	_	_	_	KI 1/11	
1	12	404	18	394	18	0.632	30.6	LOSIC	13.8	97 9	0 78	0 79	0 78	76
2	 T1	136	1.6	132	1.5	0.314	21.9	LOSB	8.6	61.2	0.67	0.65	0.67	31.9
3	R2	105	2.0	102	2.0	0.314	26.2	LOSB	8.6	61.2	0.67	0.65	0.67	31.6
Appre	hach	645	1.0	620 ^{N1}	1.0	0.632	20.2		12.9	07.0	0.07	0.00	0.07	20.2
Appro	acri	045	1.0	029	1.0	0.052	20.1	L03 D	13.0	97.9	0.74	0.74	0.74	20.2
East:	High S	t (E)												
4	L2	35	1.8	35	1.8	0.268	35.2	LOS C	7.2	51.2	0.77	0.66	0.77	27.0
5	T1	312	2.0	312	2.0	0.268	30.6	LOS C	7.4	52.6	0.77	0.65	0.77	27.3
6	R2	31	2.0	31	2.0	0.124	42.9	LOS D	1.4	10.1	0.81	0.72	0.81	31.3
Appro	bach	377	2.0	377	2.0	0.268	32.0	LOS C	7.4	52.6	0.77	0.65	0.77	27.8
N la utila	· \ \ /	Ct (NI)												
	: vvortr		~ ~			0.044	47.0		0.4		0.47	0.04	0.47	40.0
1	L2	1	2.0	1	2.0	0.014	17.9	LOSB	0.4	2.9	0.47	0.34	0.47	42.2
8	T1	14	2.0	14	2.0	0.014	13.4	LOS A	0.4	2.9	0.47	0.34	0.47	36.6
9	R2	16	2.0	16	2.0	0.045	21.4	LOS B	0.4	3.1	0.70	0.65	0.70	31.8
Appro	bach	31	2.0	31	2.0	0.045	17.7	LOS B	0.4	3.1	0.59	0.50	0.59	34.3
West	High	St (W)												
10	L2	118	2.0	118	2.0	0.223	36.1	LOS C	5.0	35.7	0.77	0.74	0.77	28.9
11	T1	495	2.0	495	2.0	0.378	31.9	LOS C	10.9	77.5	0.80	0.68	0.80	30.2
12	R2	221	2.0	221	2.0	0.716	49.4	LOS D	12.4	88.0	0.96	0.87	1.02	10.2
Appro	hach	834	2.0	834	2.0	0 716	37.2	105 0	12.4	88.0	0.84	0.74	0.86	25.7
· ·ppro		001	2.5	001	2.0	0.7 10	07.2	200 0		00.0	0.04	0.14	0.00	20.1
All Ve	hicles	1886	1.9	<mark>1870</mark> N1	1.9	0.716	32.7	LOS C	13.8	97.9	0.79	0.72	0.80	24.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian	of Queue Distance m	Prop. Queued	Effective Stop Rate			
P1	South Full Crossing	53	32.3	LOS D	0.1	0.1	0.73	0.73			

P2	East Full Crossing	53	18.7	LOS B	0.1	0.1	0.56	0.56
P3	North Full Crossing	53	33.8	LOS D	0.1	0.1	0.75	0.75
P3S	North Slip/Bypass Lane	53	27.4	LOS C	0.1	0.1	0.68	0.68
	Crossing							
P4	West Full Crossing	53	26.1	LOS C	0.1	0.1	0.66	0.66
P4S	West Slip/Bypass Lane	53	54.3	LOS E	0.2	0.2	0.95	0.95
	Crossing							
All Pedestrians		316	32.1	LOS D			0.72	0.72

Site: 5 [5. Worth St and Union Ln]

♦♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

Worth St and Union Ln 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Stop (Two-Way)

Movement Performance - Vehicles														
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		rato		km/h
South: Worth		n St (S)												
1	L2	76	2.8	73	2.8	0.040	3.9	LOS A	0.0	0.0	0.00	0.52	0.00	27.5
2	T1	442	2.1	426	2.1	0.221	0.0	LOS A	2.3	16.1	0.00	0.00	0.00	50.0
Appro	ach	518	2.2	<mark>499</mark> N1	2.2	0.221	0.6	NA	2.3	16.1	0.00	0.08	0.00	44.3
East: Union Ln		Ln (E)												
4	L2	18	2.0	18	2.0	0.068	4.0	LOS A	0.3	1.8	0.35	0.51	0.35	35.0
5	T1	21	1.9	21	1.9	0.068	9.0	LOS A	0.3	1.8	0.35	0.51	0.35	35.0
6	R2	25	2.0	25	2.0	0.138	11.7	LOS A	0.3	1.8	0.64	0.81	0.64	31.9
Appro	ach	64	2.0	64	2.0	0.138	8.7	LOS A	0.3	1.8	0.46	0.63	0.46	33.7
North	: Worth	n St (N)												
8	T1	143	2.0	143	2.0	0.093	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
9	R2	100	1.9	100	1.9	0.111	6.7	LOS A	0.4	2.9	0.40	0.64	0.40	22.0
Appro	ach	243	2.0	243	2.0	0.111	2.8	NA	0.4	2.9	0.17	0.26	0.17	32.5
West:	Union	Ln (W)												
10	L2	76	0.0	76	0.0	0.169	6.1	LOS A	0.7	4.8	0.54	0.72	0.54	19.7
12	R2	32	0.0	32	0.0	0.169	12.6	LOS A	0.7	4.8	0.54	0.72	0.54	19.7
Appro	ach	107	0.0	107	0.0	0.169	8.0	LOS A	0.7	4.8	0.54	0.72	0.54	19.7
All Ve	hicles	933	1.9	<mark>913</mark> N1	1.9	0.221	2.6	NA	2.3	16.1	0.14	0.24	0.14	34.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 6 [6. Worth St and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

Worth St and Union Rd 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 130 seconds (Site User-Given Cycle Time)

Move	vement Performance - Vehicles / Turn Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No.Average													
Mov	Turn	Demand F	lows	Arrival I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rate		km/h
South	n: Worth	n St (S)	70	Voli/II	,,,				Von					
1	L2	22	2.0	22	2.0	0.390	68.8	LOS E	3.5	24.6	0.99	0.75	0.99	17.8
2	T1	20	2.0	20	2.0	0.390	64.2	LOS E	3.5	24.6	0.99	0.75	0.99	17.8
3	R2	13	2.0	13	2.0	0.390	68.8	LOS E	3.5	24.6	0.99	0.75	0.99	25.8
Appro	bach	55	2.0	55	2.0	0.390	67.2	LOS E	3.5	24.6	0.99	0.75	0.99	20.1
East:	Union	Rd (E)												
4	L2	4	2.0	4	2.0	0.145	13.6	LOS A	4.2	29.9	0.41	0.35	0.41	44.1
5	T1	174	2.0	174	2.0	0.145	9.0	LOS A	4.2	29.9	0.41	0.35	0.41	40.2
6	R2	423	2.0	423	2.0	0.743	23.3	LOS B	18.0	127.9	0.71	0.80	0.72	30.7
Appro	bach	601	2.0	601	2.0	0.743	19.1	LOS B	18.0	127.9	0.62	0.67	0.63	33.1
North	: Worth	St (N)												
7	L2	131	0.0	131	0.0	0.699	68.3	LOS E	8.5	59.2	1.00	0.84	1.09	18.6
8	T1	8	0.0	8	0.0	0.068	54.7	LOS D	0.9	6.3	0.91	0.67	0.91	20.9
9	R2	7	0.0	7	0.0	0.068	58.6	LOS E	0.9	6.3	0.91	0.67	0.91	3.8
Appro	bach	146	0.0	146	0.0	0.699	67.0	LOS E	8.5	59.2	0.99	0.82	1.07	18.3
West	: Union	Rd (W)												
10	L2	111	1.9	90	1.9	0.068	7.4	LOS A	1.4	9.6	0.25	0.58	0.25	19.7
11	T1	289	2.2	236	2.2	0.210	9.3	LOS A	5.8	41.3	0.42	0.37	0.42	40.6
12	R2	5	0.0	4	0.0	0.210	13.2	LOS A	5.8	41.3	0.42	0.37	0.42	39.6
Appro	bach	405	2.1	<mark>330</mark> N1	2.1	0.210	8.8	LOS A	5.8	41.3	0.37	0.43	0.37	38.9
All Ve	hicles	1207	1.8	<mark>1132</mark> N1	1.9	0.743	24.6	LOS B	18.0	127.9	0.61	0.62	0.63	29.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate					
		ped/h	sec		ped	m							
P1	South Full Crossing	53	9.6	LOS A	0.1	0.1	0.39	0.39					

P2	East Full Crossing	53	57.4	LOS E	0.2	0.2	0.94	0.94
P3	North Full Crossing	53	11.7	LOS B	0.1	0.1	0.42	0.42
P4	West Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96
All P	edestrians	211	34.5	LOS D			0.68	0.68

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: UrbApart [Urban Apartments]

♦♦ Network: N101 [Network Model - 2026 Future Base AM Peak (No Link Rd, With Urban Apt)]

Urban Apartments 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Bacł Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	ast: Union Lane (E)													
6	R2	6	0.0	6	0.0	0.003	5.9	LOS A	0.0	0.0	0.00	0.74	0.00	41.5
Appro	bach	6	0.0	6	0.0	0.003	5.9	NA	0.0	0.0	0.00	0.74	0.00	41.5
North	: Urbar	n Apartment	Acce	SS										
7	L2	56	0.0	56	0.0	0.030	8.0	LOS A	0.0	0.0	0.00	1.00	0.00	47.7
Appro	bach	56	0.0	56	0.0	0.030	8.0	LOS A	0.0	0.0	0.00	1.00	0.00	47.7
All Ve	hicles	62	0.0	62	0.0	0.030	7.8	NA	0.0	0.0	0.00	0.97	0.00	46.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 1 [1. High St and Mulgoa Rd]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

High Street and Mulgoa Road 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Move	rement Performance - Vehicles Turn Demand Flows Arrival Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No Average													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	0/,	veh/h	0/	vic	202		veh	m		Rate		km/h
South	n: Mula	ba Road (S))	VEII/II	70	v/C	360		Ven		_			NI11/11
1	L2	246	2.0	246	2.0	0.384	27.9	LOS B	9.2	65.6	0.80	0.78	0.80	35.0
2	T1	1014	2.0	1014	2.0	0.777	57.5	LOS E	22.4	159.7	1.00	0.90	1.06	24.9
3	R2	73	2.0	73	2.0	0.278	64.1	LOS E	4.5	32.0	0.93	0.76	0.93	8.6
Appro	bach	1333	2.0	1333	2.0	0.777	52.4	LOS D	22.4	159.7	0.96	0.87	1.00	25.6
East:	High S	treet (E)												
4	L2	127	2.5	127	2.5	0.180	35.7	LOS C	5.6	40.2	0.70	0.74	0.70	11.3
5	T1	566	2.0	563	2.0	0.769	59.2	LOS E	20.0	142.5	0.99	0.89	1.06	23.8
6	R2	365	2.0	363	2.0	0.772	73.7	LOS F	12.7	90.7	1.00	0.88	1.12	20.6
Appro	bach	1059	2.1	1053 ^{N1}	2.1	0.772	61.3	LOS E	20.0	142.5	0.96	0.87	1.04	21.8
North	: Castle	ereagh Roa	d (N)											
7	L2	167	2.0	167	2.0	0.125	7.6	LOS A	1.8	12.6	0.25	0.62	0.25	48.3
8	T1	1403	2.0	1403	2.0	0.666	40.5	LOS C	26.7	190.1	0.91	0.80	0.91	26.4
9	R2	667	2.0	667	2.0	0.773	36.7	LOS C	13.7	97.7	0.99	0.87	1.05	37.6
Appro	bach	2238	2.0	2238	2.0	0.773	36.9	LOS C	26.7	190.1	0.88	0.81	0.90	31.5
West	· Hiah S	Street (W)												
10	L2	654	2.0	654	2.0	0.373	18.9	LOS B	8.8	62.4	0.68	0.75	0.68	45.4
11	T1	269	2.0	269	2.0	0.346	53.3	LOS D	7.9	56.5	0.91	0.75	0.91	22.5
12	R2	162	2.0	162	2.0	0.344	66.6	LOS E	5.1	36.6	0.95	0.77	0.95	19.3
Appro	bach	1085	2.0	1085	2.0	0.373	34.6	LOS C	8.8	62.4	0.77	0.76	0.77	35.2
All Ve	hicles	5715	2.0	5709 ^{N1}	2.0	0.777	44.6	LOS D	26.7	190.1	0.89	0.82	0.92	28.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate					
		ped/h	sec		ped	m							
P11	South Stage 1	53	53.3	LOS E	0.2	0.2	0.87	0.87					

P12 P1S	South Stage 2 South Slip/Bypass Lane	53 53	58.6 10.9	LOS E LOS B	0.2 0.1	0.2 0.1	0.92 0.55	0.92 0.55
P2 P2S	East Full Crossing East Slip/Bypass Lane Crossing	53 53	41.7 16.6	LOS E LOS B	0.2 0.1	0.2 0.1	0.77 0.49	0.77 0.49
P31 P32 P3S	North Stage 1 North Stage 2 North Slip/Bypass Lane Crossing	53 53 53	55.9 27.0 0.5	LOS E LOS C LOS A	0.2 0.1 0.0	0.2 0.1 0.0	0.89 0.87 0.11	0.89 0.87 0.11
P4 P4S	West Full Crossing West Slip/Bypass Lane Crossing	53 53	57.7 20.5	LOS E LOS C	0.2 0.1	0.2 0.1	0.91 0.71	0.91 0.71
All Pe	destrians	526	34.3	LOS D			0.71	0.71

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: 2 [2. Mulgoa Rd and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

Mulgoa Rd and Union Rd 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Giveway / Yield (Two-Way)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Mulgo	ba Road (S))											
2	T1	1324	2.0	1324	2.0	0.388	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
3	R2	159	2.0	159	2.0	0.729	39.2	LOS C	3.9	27.8	0.94	1.21	1.86	26.5
Appro	ach	1483	2.0	1483	2.0	0.729	4.3	NA	3.9	27.8	0.10	0.13	0.20	52.7
East:	Union	Road (E)												
4	L2	167	2.5	167	2.5	0.210	7.0	LOS A	0.8	5.7	0.45	0.68	0.45	46.8
Appro	ach	167	2.5	167	2.5	0.210	7.0	LOS A	0.8	5.7	0.45	0.68	0.45	46.8
North:	Mulgo	a Road (N)												
7	L2	116	2.0	116	2.0	0.063	5.6	LOS A	0.1	0.4	0.00	0.58	0.00	38.5
8	T1	1591	2.0	1590	2.0	0.275	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	1706	2.0	1706	2.0	0.275	0.4	NA	0.1	0.4	0.00	0.04	0.00	59.3
All Ve	hicles	3357	2.0	<mark>3356</mark> N	¹ 2.0	0.729	2.4	NA	3.9	27.8	0.07	0.11	0.11	55.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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V Site: 3 [3. High St and Civic Roundabout]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

High and Civic Roundabout 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Roundabout

Move	ement	Performa	nce -	Vehicle	es									
Mov ID	Turn	Demand I Total	Flows HV	Arrival I Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	High S	t (E)												
5	T1	959	2.0	953	2.0	0.420	2.4	LOS A	1.9	13.5	0.20	0.29	0.20	40.6
6	R2	28	0.0	28	0.0	0.420	7.8	LOS A	1.9	13.5	0.20	0.30	0.20	50.5
Appro	bach	987	1.9	<mark>982</mark> ^{N1}	1.9	0.420	2.6	LOS A	1.9	13.5	0.20	0.29	0.20	41.3
North	: Civic I	PI (N)												
7	L2	52	2.0	52	2.0	0.158	3.7	LOS A	0.6	4.5	0.42	0.62	0.42	42.1
9	R2	100	2.0	100	2.0	0.158	8.9	LOS A	0.6	4.5	0.42	0.62	0.42	42.1
Appro	bach	152	2.0	152	2.0	0.158	7.1	LOS A	0.6	4.5	0.42	0.62	0.42	42.1
West	: High S	St (W)												
10	L2	74	2.0	74	2.0	0.193	2.6	LOS A	0.9	6.7	0.09	0.26	0.09	48.0
11	T1	452	2.0	452	2.0	0.193	2.1	LOS A	0.9	6.7	0.09	0.25	0.09	40.7
Appro	bach	525	2.0	525	2.0	0.193	2.2	LOS A	0.9	6.7	0.09	0.25	0.09	43.3
All Ve	hicles	1664	2.0	<mark>1658</mark> ^{N1}	2.0	0.420	2.9	LOS A	1.9	13.5	0.19	0.31	0.19	42.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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PM Peak - TBC\200623 - East DA Scheme - 2026 Future Base (No Link Rd, With Urban Apt) - PM Peak.sip8

Site: 4 [4. High St and Worth St]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

High and Worth 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Mov	ement	Performa	nce -	Vehicle	s									
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rato	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Tale		km/h
South	n: Worth	n St (S)												
1	L2	211	2.0	204	2.0	0.726	58.8	LOS E	11.9	84.7	1.00	0.86	1.08	4.2
2	T1	111	1.9	107	1.9	0.450	47.9	LOS D	7.4	52.2	0.94	0.77	0.94	23.1
3	R2	33	0.0	32	0.0	0.450	52.1	LOS D	7.4	52.2	0.94	0.77	0.94	22.9
Appro	bach	354	1.8	<mark>342</mark> N1	1.8	0.726	54.8	LOS D	11.9	84.7	0.97	0.82	1.02	13.5
East:	High S	t (E)												
4	L2	103	2.0	103	2.0	0.526	32.1	LOS C	13.7	97.5	0.79	0.73	0.79	28.1
5	T1	632	2.0	632	2.0	0.526	26.7	LOS B	17.6	125.5	0.78	0.70	0.78	28.9
6	R2	211	2.0	211	2.0	0.458	33.3	LOS C	9.1	65.1	0.78	0.79	0.78	34.1
Appro	bach	945	2.0	945	2.0	0.526	28.8	LOS C	17.6	125.5	0.78	0.72	0.78	30.5
North	: Worth	St (N)												
7	L2	1	0.0	1	0.0	0.421	28.3	LOS B	7.5	53.3	0.71	0.60	0.71	37.8
8	T1	184	2.0	184	2.0	0.421	23.8	LOS B	7.5	53.3	0.71	0.60	0.71	30.6
9	R2	309	2.0	309	2.0	0.599	29.7	LOS C	11.8	83.9	0.91	0.82	0.91	27.9
Appro	bach	495	2.0	495	2.0	0.599	27.5	LOS B	11.8	83.9	0.84	0.74	0.84	28.9
West	: High S	St (W)												
10	L2	203	2.0	203	2.0	0.471	41.7	LOS C	9.8	69.7	0.87	0.80	0.87	27.1
11	T1	255	2.0	255	2.0	0.153	21.8	LOS B	4.4	31.7	0.64	0.52	0.64	34.5
12	R2	232	2.0	232	2.0	1.117	194.3	LOS F	28.9	205.4	1.00	1.48	2.31	3.0
Appro	bach	689	2.0	689	2.0	1.117	85.6	LOS F	28.9	205.4	0.83	0.92	1.27	14.8
All Ve	hicles	2483	2.0	<mark>2472</mark> ^{N1}	2.0	1.117	48.0	LOS D	28.9	205.4	0.83	0.80	0.96	21.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	53	24.8	LOS C	0.1	0.1	0.64	0.64					

P2	East Full Crossing	53	25.4	LOS C	0.1	0.1	0.65	0.65
P3	North Full Crossing	53	26.1	LOS C	0.1	0.1	0.66	0.66
P3S	North Slip/Bypass Lane	53	20.5	LOS C	0.1	0.1	0.58	0.58
	Crossing							
P4	West Full Crossing	53	52.4	LOS E	0.2	0.2	0.94	0.94
P4S	West Slip/Bypass Lane	53	36.9	LOS D	0.1	0.1	0.79	0.79
	Crossing							
All Pedestrians		316	31.0	LOS D			0.71	0.71

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 5 [5. Worth St and Union Ln]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

Worth St and Union Ln 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicle	s									
Mov	Turn	Demand	Flows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		, tato		km/h
South	: Wortl	n St (S)												
1	L2	75	1.5	72	1.5	0.039	3.9	LOS A	0.0	0.0	0.00	0.52	0.00	27.5
2	T1	311	2.0	299	2.0	0.156	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
Appro	ach	385	1.9	371 ^{N1}	1.9	0.156	0.7	NA	0.0	0.0	0.00	0.10	0.00	42.7
East:	Union	Ln (E)												
4	L2	18	0.0	18	0.0	0.106	4.6	LOS A	0.3	2.1	0.51	0.62	0.51	34.5
5	T1	26	0.0	26	0.0	0.106	9.4	LOS A	0.3	2.1	0.51	0.62	0.51	34.5
6	R2	25	0.0	25	0.0	0.100	10.7	LOS A	0.2	1.6	0.61	0.79	0.61	32.5
Appro	ach	69	0.0	69	0.0	0.106	8.6	LOS A	0.3	2.1	0.54	0.68	0.54	33.7
North	: Worth	n St (N)												
8	T1	382	2.0	368	2.0	0.149	0.3	LOS A	13.8	97.9	0.07	0.08	0.07	42.4
9	R2	116	1.5	112	1.5	0.149	6.0	LOS A	0.7	5.1	0.30	0.34	0.30	27.9
Appro	ach	498	1.9	481 ^{N1}	1.9	0.149	1.7	NA	13.8	97.9	0.12	0.14	0.12	37.7
West:	Union	Ln (W)												
10	L2	2	0.0	2	0.0	0.091	5.2	LOS A	0.3	2.4	0.63	0.82	0.63	14.4
12	R2	32	0.0	32	0.0	0.091	12.6	LOS A	0.3	2.4	0.63	0.82	0.63	14.4
Appro	ach	34	0.0	34	0.0	0.091	12.1	LOS A	0.3	2.4	0.63	0.82	0.63	14.4
All Ve	hicles	986	1.7	<mark>955</mark> N1	1.8	0.156	2.2	NA	13.8	97.9	0.12	0.19	0.12	35.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 6 [6. Worth St and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

Worth St and Union Rd 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 130 seconds (Site User-Given Cycle Time)

Move	ovement Performance - Vehicles													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	vic	202		veh	m		Rate		km/h
South	n: Worth	n St (S)	/0	ven/n	/0	V/C	360		Ven		_			KI11/11
1	L2	16	0.0	16	0.0	0.329	68.2	LOS E	3.0	21.2	0.98	0.74	0.98	18.1
2	T1	28	0.0	28	0.0	0.329	63.6	LOS E	3.0	21.2	0.98	0.74	0.98	18.1
3	R2	4	0.0	4	0.0	0.329	68.2	LOS E	3.0	21.2	0.98	0.74	0.98	26.1
Appro	bach	48	0.0	48	0.0	0.329	65.5	LOS E	3.0	21.2	0.98	0.74	0.98	19.0
East:	Union	Rd (E)												
4	L2	33	0.0	33	0.0	0.190	22.9	LOS B	6.1	43.2	0.58	0.53	0.58	39.3
5	T1	147	2.1	147	2.1	0.190	18.3	LOS B	6.1	43.2	0.58	0.53	0.58	33.0
6	R2	360	2.0	360	2.0	1.058	151.5	LOS F	42.2	300.8	1.00	1.30	1.89	9.7
Appro	bach	540	1.9	540	1.9	1.058	107.4	LOS F	42.2	300.8	0.86	1.04	1.45	13.2
North	: Worth	St (N)												
7	L2	421	2.0	409	2.0	0.903	69.6	LOS E	11.5	81.6	1.00	0.99	1.25	18.3
8	T1	32	0.0	31	0.0	0.068	37.6	LOS C	1.6	11.3	0.78	0.60	0.78	25.8
9	R2	4	0.0	4	0.0	0.068	41.4	LOS C	1.6	11.3	0.78	0.60	0.78	5.4
Appro	bach	457	1.8	<mark>444</mark> N1	1.8	0.903	67.1	LOS E	11.5	81.6	0.98	0.96	1.21	18.7
vvest:	Union	Rd (W)												
10	L2	153	2.1	153	2.1	0.146	13.3	LOS A	4.0	28.5	0.42	0.63	0.42	13.4
11	T1	316	2.0	316	2.0	0.450	19.7	LOS B	11.5	81.6	0.63	0.56	0.63	33.6
12	R2	11	0.0	11	0.0	0.450	24.0	LOS B	11.5	81.6	0.64	0.56	0.64	32.8
Appro	bach	479	2.0	479	2.0	0.450	17.7	LOS B	11.5	81.6	0.56	0.58	0.56	31.1
All Ve	hicles	1524	1.9	<mark>1511</mark> ^{N1}	1.9	1.058	65.8	LOS E	42.2	300.8	0.80	0.86	1.08	17.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/b	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	18.4	LOS B	0.1	0.1	0.53	0.53						

P2	East Full Crossing	53	40.9	LOS E	0.2	0.2	0.79	0.79
P3	North Full Crossing	53	21.1	LOS C	0.1	0.1	0.57	0.57
P4	West Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96
All P	edestrians	211	34.9	LOS D			0.71	0.71

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: UrbApart [Urban Apartments]

♦♦ Network: N101 [Network Model - 2026 Future Base PM Peak (No Link Rd, With Urban Apt)]

Urban Apartments 2026 Future Base No Link Road, With Urban Apartments Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicl	es									
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		, tato		km/h
East: Union Lane (E)														
6	R2	49	0.0	49	0.0	0.026	5.9	LOS A	0.0	0.0	0.00	0.74	0.00	41.5
Appro	bach	49	0.0	49	0.0	0.026	5.9	NA	0.0	0.0	0.00	0.74	0.00	41.5
North	: Urban	Apartment	Acce	ss										
7	L2	13	0.0	13	0.0	0.007	8.0	LOS A	0.0	0.0	0.00	1.00	0.00	47.7
Appro	bach	13	0.0	13	0.0	0.007	8.0	LOS A	0.0	0.0	0.00	1.00	0.00	47.7
All Ve	hicles	62	0.0	<mark>61</mark> ^N	0.0	0.026	6.4	NA	0.0	0.0	0.00	0.80	0.00	42.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 1 [1. High St and Mulgoa Rd]

High Street and Mulgoa Road East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Mov	ovement Performance - Vehicles													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rale		km/h
Sout	h: Mulgo	oa Road (S)												
1	L2	158	2.0	158	2.0	0.224	23.4	LOS B	5.0	35.2	0.71	0.74	0.71	37.5
2	T1	1131	2.0	1131	2.0	0.707	49.4	LOS D	23.2	165.0	0.96	0.83	0.96	27.2
3	R2	111	1.9	111	1.9	0.497	69.0	LOS E	7.2	51.4	0.98	0.79	0.98	8.0
Appro	oach	1399	2.0	1399	2.0	0.707	48.0	LOS D	23.2	165.0	0.94	0.82	0.94	26.6
East:	High S	treet (E)												
4	L2	49	2.1	49	2.1	0.080	39.2	LOS C	2.2	16.0	0.71	0.71	0.71	10.5
5	T1	222	1.4	219	1.4	0.328	55.2	LOS D	6.6	46.9	0.92	0.74	0.92	24.8
6	R2	148	1.4	147	1.4	0.349	68.6	LOS E	4.7	33.5	0.96	0.77	0.96	21.6
Appro	oach	420	1.5	<mark>415</mark> ^{N1}	1.5	0.349	58.0	LOS E	6.6	46.9	0.91	0.74	0.91	22.6
North	n: Castle	ereagh Road	d (N)											
7	L2	288	1.9	288	1.9	0.229	10.5	LOS A	5.4	38.2	0.37	0.66	0.37	45.0
8	T1	1200	2.0	1200	2.0	0.483	31.2	LOS C	19.6	139.2	0.78	0.68	0.78	30.3
9	R2	593	2.0	593	2.0	0.708	33.7	LOS C	10.4	74.1	0.98	0.84	0.98	38.7
Appro	oach	2081	2.0	2081	2.0	0.708	29.0	LOS C	19.6	139.2	0.78	0.72	0.78	34.8
West	: High S	Street (W)												
10	L2	887	2.0	887	2.0	0.547	23.7	LOS B	14.7	104.8	0.79	0.81	0.81	42.8
11	T1	478	2.0	478	2.0	0.717	61.9	LOS E	15.8	112.5	1.00	0.86	1.03	20.5
12	R2	303	1.9	303	1.9	0.724	73.6	LOS F	10.5	74.8	1.00	0.85	1.09	18.0
Appro	oach	1668	2.0	1668	2.0	0.724	43.7	LOS D	15.8	112.5	0.89	0.83	0.92	31.1
All Ve	ehicles	5568	2.0	<mark>5563^{N1}</mark>	2.0	0.724	40.4	LOS C	23.2	165.0	0.86	0.78	0.87	30.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m								
P11	South Stage 1	53	56.8	LOS E	0.2	0.2	0.90	0.90						
P12	South Stage 2	53	61.4	LOS F	0.2	0.2	0.94	0.94						

P1S	South Slip/Bypass Lane Crossing	53	12.9	LOS B	0.1	0.1	0.59	0.59
P2	East Full Crossing	53	35.1	LOS D	0.1	0.1	0.71	0.71
P2S	East Slip/Bypass Lane	53	13.3	LOS B	0.1	0.1	0.44	0.44
	Crossing							
P31	North Stage 1	53	59.6	LOS E	0.2	0.2	0.92	0.92
P32	North Stage 2	53	26.8	LOS C	0.1	0.1	0.87	0.87
P3S	North Slip/Bypass Lane	53	0.5	LOS A	0.0	0.0	0.11	0.11
	Crossing							
P4	West Full Crossing	53	51.5	LOS E	0.2	0.2	0.86	0.86
P4S	West Slip/Bypass Lane	53	17.6	LOS B	0.1	0.1	0.67	0.67
	Crossing							
All Peo	destrians	526	33.6	LOS D			0.70	0.70

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: 2 [2. Mulgoa Rd and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Mulgoa Rd and Union Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	nce -	Vehic	les									
Mov ID	Turn	Demand F Total	lows= HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Mulgo	ba Road (S))											
2	T1	1404	2.0	1404	2.0	0.660	5.5	LOS A	6.7	47.7	0.22	0.00	0.36	50.9
3	R2	374	2.0	374	2.0	1.613	580.2	LOS F	96.5	686.6	1.00	5.50	18.42	3.0
Appro	ach	1778	2.0	1778	2.0	1.613	126.3	NA	96.5	686.6	0.38	1.16	4.16	11.6
East:	Union	Road (E)												
4	L2	348	1.5	348	1.5	0.387	7.0	LOS A	2.4	16.8	0.55	0.74	0.64	46.8
Appro	ach	348	1.5	348	1.5	0.387	7.0	LOS A	2.4	16.8	0.55	0.74	0.64	46.8
North:	Mulgo	a Road (N)												
7	L2	351	1.8	350	1.8	0.191	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	38.5
8	T1	1252	2.0	1251	2.0	0.217	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
Appro	ach	1602	2.0	1602	2.0	0.217	1.2	NA	0.0	0.0	0.00	0.13	0.00	57.8
All Ve	hicles	3728	1.9	3728	1.9	1.613	61.4	NA	96.5	686.6	0.23	0.67	2.04	20.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not

a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 3 [3. High St and Civic Roundabout]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

High and Civic Roundabout East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Roundabout

Move	lovement Performance - Vehicles													
Mov ID	Turn	Demand F Total	lows= HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective / Stop	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Tato		km/h
South	n: Link I	Rd (S)												
1	L2	94	0.0	93	0.0	0.173	2.9	LOS A	0.8	5.3	0.38	0.57	0.38	27.8
2	T1	4	0.0	4	0.0	0.173	2.8	LOS A	0.8	5.3	0.38	0.57	0.38	47.3
3	R2	94	0.0	93	0.0	0.173	7.9	LOS A	0.8	5.3	0.38	0.57	0.38	27.8
Appro	bach	192	0.0	<mark>190</mark> N1	0.0	0.173	5.4	LOS A	0.8	5.3	0.38	0.57	0.38	29.2
East:	High S	t (E)												
5	T1	309	0.0	305	0.0	0.109	2.1	LOS A	0.5	3.2	0.07	0.27	0.07	41.9
6	R2	29	0.0	29	0.0	0.109	7.5	LOS A	0.4	3.1	0.07	0.32	0.07	50.6
Appro	bach	339	0.0	<mark>334</mark> N1	0.0	0.109	2.5	LOS A	0.5	3.2	0.07	0.27	0.07	43.8
North	: Civic	PI (N)												
7	L2	12	0.0	12	0.0	0.036	4.6	LOS A	0.2	1.2	0.54	0.64	0.54	41.6
9	R2	21	0.0	21	0.0	0.036	9.8	LOS A	0.2	1.2	0.54	0.64	0.54	41.6
Appro	bach	33	0.0	33	0.0	0.036	8.0	LOS A	0.2	1.2	0.54	0.64	0.54	41.6
West	: High S	St (W)												
10	L2	74	0.0	74	0.0	0.309	3.0	LOS A	1.5	10.8	0.21	0.30	0.21	47.3
11	T1	787	0.0	787	0.0	0.309	2.5	LOS A	1.5	10.8	0.22	0.30	0.22	38.5
Appro	bach	861	0.0	861	0.0	0.309	2.6	LOS A	1.5	10.8	0.22	0.30	0.22	40.6
All Ve	hicles	1424	0.0	<mark>1418</mark> ^{N1}	0.0	0.309	3.1	LOS A	1.5	10.8	0.21	0.34	0.21	40.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 4 [4. High St and Worth St]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

High and Worth East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Move	ovement Performance - Vehicles													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back of	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles L	Jistance	Queued	Stop Rate	Cycles S	peed
		veh/h	%	veh/h	%	v/c	sec		veh	m		rato		km/h
South	n: Worth	n St (S)												
1	L2	384	2.0	374	2.0	0.636	32.6	LOS C	13.7	97.9	0.80	0.80	0.80	7.2
2	T1	134	1.9	130	2.0	0.328	24.0	LOS B	8.9	63.2	0.70	0.67	0.70	30.9
3	R2	104	2.0	101	2.1	0.328	28.3	LOS B	8.9	63.2	0.70	0.67	0.70	30.6
Appro	bach	622	2.0	605 ^{N1}	2.0	0.636	30.0	LOS C	13.7	97.9	0.76	0.75	0.76	19.6
East:	High S	t (E)												
4	L2	35	1.8	35	1.8	0.257	32.9	LOS C	7.1	50.8	0.74	0.64	0.74	28.0
5	T1	322	1.9	322	1.9	0.257	28.3	LOS B	7.3	52.1	0.74	0.63	0.74	28.2
6	R2	31	2.0	31	2.0	0.122	41.2	LOS C	1.4	9.8	0.79	0.72	0.79	31.7
Appro	bach	387	1.9	387	1.9	0.257	29.7	LOS C	7.3	52.1	0.75	0.64	0.75	28.6
North	: Worth	n St (N)												
7	L2	1	2.0	1	2.0	0.015	19.5	LOS B	0.4	3.1	0.49	0.35	0.49	41.5
8	T1	14	2.0	14	2.0	0.015	15.0	LOS B	0.4	3.1	0.49	0.35	0.49	35.5
9	R2	16	2.0	16	2.0	0.047	23.0	LOS B	0.5	3.3	0.72	0.65	0.72	31.0
Appro	bach	31	2.0	31	2.0	0.047	19.3	LOS B	0.5	3.3	0.61	0.51	0.61	33.4
West	High S	St (W)												
10	L2	118	2.0	118	2.0	0.208	33.8	LOS C	4.8	34.2	0.74	0.73	0.74	29.7
11	T1	541	1.8	541	1.8	0.385	29.9	LOS C	11.6	82.3	0.79	0.67	0.79	31.0
12	R2	237	1.9	237	1.9	0.716	47.2	LOS D	13.0	92.6	0.95	0.87	1.00	10.5
Appro	bach	896	1.9	<mark>895</mark> ^{N1}	1.9	0.716	35.0	LOS C	13.0	92.6	0.82	0.73	0.84	26.4
All Ve	hicles	1936	1.9	<mark>1918</mark> N1	1.9	0.716	32.1	LOS C	13.7	97.9	0.78	0.71	0.79	25.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m								
P1	South Full Crossing	53	30.2	LOS D	0.1	0.1	0.71	0.71						
P2	East Full Crossing	53	20.5	LOS C	0.1	0.1	0.58	0.58						

P3 P3S	North Full Crossing North Slip/Bypass Lane Crossing	53 53	31.6 25.4	LOS D LOS C	0.1 0.1	0.1 0.1	0.73 0.65	0.73 0.65
P4 P4S	West Full Crossing West Slip/Bypass Lane Crossing	53 53	28.1 54.3	LOS C LOS E	0.1 0.2	0.1 0.2	0.68 0.95	0.68 0.95
All Pedestrians		316	31.7	LOS D			0.72	0.72

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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🎟 Site: 5 [5. Worth St and Union Ln]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Worth St and Union Ln East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicl	es									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Worth	n St (S)												
1	L2	76	1.9	73	2.0	0.040	3.9	LOS A	0.0	0.0	0.00	0.52	0.00	27.5
2	T1	458	1.9	441	2.0	0.229	0.0	LOS A	2.3	16.6	0.00	0.00	0.00	50.0
Appro	ach	534	1.9	<mark>514</mark> ^N	¹ 2.0	0.229	0.6	NA	2.3	16.6	0.00	0.07	0.00	44.4
East:	Union I	_n (E)												
4	L2	18	2.0	18	2.0	0.067	4.0	LOS A	0.3	1.8	0.38	0.52	0.38	34.8
5	T1	21	1.9	21	1.9	0.067	9.5	LOS A	0.3	1.8	0.38	0.52	0.38	34.8
6	R2	25	2.0	25	2.0	0.126	10.6	LOS A	0.2	1.7	0.60	0.79	0.60	32.5
Appro	ach	64	2.0	64	2.0	0.126	8.4	LOS A	0.3	1.8	0.47	0.63	0.47	33.9
North	: Worth	St (N)												
8	T1	159	1.8	159	1.8	0.093	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
9	R2	100	1.9	100	1.9	0.113	6.8	LOS A	0.4	2.9	0.41	0.65	0.41	21.5
Appro	ach	259	1.9	259	1.9	0.113	2.6	NA	0.4	2.9	0.16	0.25	0.16	32.9
All Ve	hicles	857	1.9	<mark>837</mark> N	¹ 2.0	0.229	1.8	NA	2.3	16.6	0.09	0.17	0.09	37.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 6 [6. Worth St and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Worth St and Union Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 130 seconds (Site User-Given Cycle Time)

Move	ement	Performa	nce -	Vehicle	es									
Mov	Turn	Demand I	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Iotal	ΗV	Iotal	ΗV	Sath	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Worth	n St (S)												
1	L2	22	2.0	22	2.0	0.390	68.8	LOS E	3.5	24.6	0.99	0.75	0.99	17.8
2	T1	20	2.0	20	2.0	0.390	64.2	LOS E	3.5	24.6	0.99	0.75	0.99	17.8
3	R2	13	2.0	13	2.0	0.390	68.8	LOS E	3.5	24.6	0.99	0.75	0.99	25.8
Appro	bach	55	2.0	55	2.0	0.390	67.2	LOS E	3.5	24.6	0.99	0.75	0.99	20.1
East:	Union	Rd (E)												
4	L2	4	2.0	4	2.0	0.142	12.8	LOS A	4.0	28.5	0.39	0.34	0.39	44.5
5	T1	174	2.0	174	2.0	0.142	8.2	LOS A	4.0	28.5	0.39	0.34	0.39	40.9
6	R2	423	2.0	423	2.0	0.772	25.8	LOS B	19.2	137.0	0.72	0.82	0.76	29.5
Appro	bach	601	2.0	601	2.0	0.772	20.6	LOS B	19.2	137.0	0.63	0.68	0.65	32.2
North	: Worth	n St (N)												
7	L2	116	0.0	116	0.0	0.733	71.1	LOS F	7.7	53.7	1.00	0.85	1.14	18.1
8	T1	7	0.0	7	0.0	0.148	57.8	LOS E	1.7	12.1	0.94	0.71	0.94	20.1
9	R2	22	0.0	22	0.0	0.148	61.6	LOS E	1.7	12.1	0.94	0.71	0.94	3.6
Appro	bach	145	0.0	145	0.0	0.733	69.0	LOS E	7.7	53.7	0.99	0.82	1.10	16.8
West	: Union	Rd (W)												
10	L2	121	1.7	100	2.1	0.081	8.0	LOS A	1.7	12.0	0.27	0.57	0.27	19.1
11	T1	336	1.9	278	2.3	0.248	8.6	LOS A	6.6	47.0	0.41	0.37	0.41	41.1
12	R2	6	0.0	5	0.0	0.248	12.6	LOS A	6.6	47.0	0.42	0.37	0.42	40.1
Appro	bach	463	1.8	<mark>383</mark> ^{N1}	2.2	0.248	8.5	LOS A	6.6	47.0	0.37	0.42	0.37	39.3
All Ve	hicles	1264	1.7	<mark>1184</mark> ^{N1}	1.8	0.772	24.8	LOS B	19.2	137.0	0.61	0.62	0.63	29.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestr	ians						
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P1	South Full Crossing	53	8.9	LOS A	0.1	0.1	0.37	0.37
P2	East Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96

P3	North Full Crossing	53	10.8	LOS B	0.1	0.1	0.41	0.41
P4	West Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96
All Peo	destrians	211	34.6	LOS D			0.67	0.67

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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🏧 Site: 7 [7. Union Rd and Link Rd]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Union Rd and Link Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicle	S									
Mov ID	Turn	Demand F Total	lows= HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	Union	Rd (E)												
5	T1	206	0.0	206	0.0	0.119	0.3	LOS A	0.1	0.9	0.08	0.03	0.08	43.4
6	R2	13	0.0	13	0.0	0.119	6.6	LOS A	0.1	0.9	0.08	0.03	0.08	43.4
Appro	ach	219	0.0	219	0.0	0.119	0.7	NA	0.1	0.9	0.08	0.03	0.08	43.4
North: Link Rd (N)														
7	L2	58	0.0	57	0.0	0.362	11.3	LOS A	1.6	11.0	0.67	1.07	0.86	13.7
9	R2	134	0.0	133	0.0	0.362	13.9	LOS A	1.6	11.0	0.67	1.07	0.86	13.7
Appro	ach	192	0.0	<mark>190</mark> ^{N1}	0.0	0.362	13.1	LOS A	1.6	11.0	0.67	1.07	0.86	13.7
West:	Union	Rd (W)												
10	L2	20	0.0	19	0.0	0.300	3.9	LOS A	0.0	0.0	0.00	0.02	0.00	48.6
11	T1	702	0.0	564	0.0	0.300	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	48.6
Appro	ach	722	0.0	583 ^{N1}	0.0	0.300	0.1	NA	0.0	0.0	0.00	0.02	0.00	48.6
All Ve	hicles	1133	0.0	<mark>992</mark> N1	0.0	0.362	2.7	NA	1.6	11.0	0.15	0.22	0.18	31.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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V Site: 8 [8. Union Ln and Link Rd]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Union Ln and Link Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	nce -	Vehicl	es									
Mov ID	Turn	Demand F Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Link Road (S)														
2	T1	65	0.0	65	0.0	0.033	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
Appro	bach	65	0.0	65	0.0	0.033	0.0	NA	0.0	0.0	0.00	0.00	0.00	50.0
East:	Union	Ln (E)												
4	L2	124	0.0	123	0.0	0.156	3.9	LOS A	0.6	4.4	0.14	0.50	0.14	25.2
6	R2	124	0.0	123	0.0	0.156	4.6	LOS A	0.6	4.4	0.14	0.50	0.14	25.2
Appro	bach	248	0.0	<mark>246</mark> N1	0.0	0.156	4.2	LOS A	0.6	4.4	0.14	0.50	0.14	25.2
All Ve	hicles	314	0.0	311 ^{N1}	0.0	0.156	3.3	NA	0.6	4.4	0.11	0.40	0.11	26.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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🎟 Site: Drwy1 [Driveway 1]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Driveway 1 East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicle	s									
Mov ID	Turn	Demand I Total	Flows HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	ver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Link I	Rd (S)												
1	L2	33	0.0	31	0.0	0.017	7.5	LOS A	0.0	0.0	0.00	0.79	0.00	20.9
2	T1	1	0.0	1	0.0	0.017	0.0	LOS A	0.0	0.0	0.00	0.79	0.00	19.8
Appro	ach	34	0.0	33 ^{N1}	0.0	0.017	7.2	NA	0.0	0.0	0.00	0.79	0.00	20.9
North: Link Rd (N)														
8	T1	124	0.0	123	0.0	0.064	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	49.1
9	R2	1	0.0	1	0.0	0.064	4.1	LOS A	0.0	0.0	0.00	0.01	0.00	14.0
Appro	ach	125	0.0	<mark>124</mark> ^{N1}	0.0	0.064	0.0	NA	0.0	0.0	0.00	0.01	0.00	46.4
West:	Drivev	vay 1												
10	L2	65	0.0	65	0.0	0.110	2.3	LOS A	0.4	2.9	0.01	1.00	0.01	9.0
12	R2	65	0.0	65	0.0	0.110	2.9	LOS A	0.4	2.9	0.01	1.00	0.01	9.0
Appro	ach	131	0.0	131	0.0	0.110	2.6	LOS A	0.4	2.9	0.01	1.00	0.01	9.0
All Ve	hicles	289	0.0	287 ^{N1}	0.0	0.110	2.0	NA	0.4	2.9	0.00	0.55	0.00	13.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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🎟 Site: Drwy2 [Driveway 2]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Driveway 2 East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicle	s									
Mov ID	Turn	Demand I Total	lows= HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective / Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	Union	Rd (E)												
5	T1	269	0.0	268	0.0	0.141	0.1	LOS A	0.0	0.3	0.02	0.01	0.02	48.2
6	R2	3	0.0	3	0.0	0.141	8.8	LOS A	0.0	0.3	0.02	0.01	0.02	17.6
Appro	ach	273	0.0	271 ^{N1}	0.0	0.141	0.2	NA	0.0	0.3	0.02	0.01	0.02	46.5
North: Driveway 2														
7	L2	5	0.0	5	0.0	0.040	5.4	LOS A	0.1	0.9	0.62	0.97	0.62	7.6
9	R2	14	0.0	14	0.0	0.040	8.8	LOS A	0.1	0.9	0.62	0.97	0.62	7.6
Appro	ach	19	0.0	19	0.0	0.040	7.8	LOS A	0.1	0.9	0.62	0.97	0.62	7.6
West:	Union	Rd (W)												
10	L2	7	0.0	7	0.0	0.300	7.5	LOS A	0.0	0.0	0.00	0.01	0.00	26.8
11	T1	717	0.0	578	0.0	0.300	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	48.5
Appro	ach	724	0.0	585 ^{N1}	0.0	0.300	0.1	NA	0.0	0.0	0.00	0.01	0.00	47.7
All Ve	hicles	1016	0.0	<mark>875</mark> ^{N1}	0.0	0.300	0.3	NA	0.1	0.9	0.02	0.03	0.02	42.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: UrbApart [Urban Apartments]

♦♦ Network: N101 [Network Model - 2026 Development AM Peak]

Urban Apartments East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	ement	Performa	nce -	Vehicle	es									
Mov ID	Turn	Demand F Total	lows= HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective / Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East: Union Lane (E)														
5	T1	191	0.0	188	0.0	0.100	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	47.6
6	R2	7	0.0	7	0.0	0.100	5.8	LOS A	0.0	0.0	0.00	0.04	0.00	47.0
Appro	bach	198	0.0	<mark>195</mark> ^{N1}	0.0	0.100	0.2	NA	0.0	0.0	0.00	0.04	0.00	47.4
North	: Urbar	n Apartment	Acces	ss										
9	R2	57	0.0	57	0.0	0.052	2.9	LOS A	0.2	1.2	0.26	0.89	0.26	9.9
Appro	bach	57	0.0	57	0.0	0.052	2.9	LOS A	0.2	1.2	0.26	0.89	0.26	9.9
All Ve	hicles	255	0.0	252 ^{N1}	0.0	0.100	0.8	NA	0.2	1.2	0.06	0.23	0.06	13.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 1 [1. High St and Mulgoa Rd]

High Street and Mulgoa Road East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site User-Given Cycle Time)

Mov	ement	Performa	nce -	Vehicle	s									
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Trate		km/h
South	n: Mulgo	oa Road (S)												
1	L2	246	2.0	246	2.0	0.384	27.9	LOS B	9.2	65.6	0.80	0.78	0.80	35.0
2	T1	1014	2.0	1014	2.0	0.777	57.5	LOS E	22.4	159.7	1.00	0.90	1.06	24.9
3	R2	91	1.6	91	1.6	0.363	65.9	LOS E	5.7	40.6	0.95	0.77	0.95	8.4
Appro	oach	1351	2.0	1351	2.0	0.777	52.7	LOS D	22.4	159.7	0.96	0.87	1.00	25.4
East:	High S	treet (E)												
4	L2	172	1.6	170	1.6	0.240	36.5	LOS C	7.8	55.0	0.72	0.75	0.72	11.1
5	T1	597	2.0	591	2.0	0.789	59.4	LOS E	21.5	153.0	0.99	0.91	1.08	23.8
6	R2	384	2.0	380	2.0	0.808	75.5	LOS F	13.6	96.9	1.00	0.91	1.17	20.3
Appro	bach	1153	1.9	<mark>1142</mark> N1	1.9	0.808	61.4	LOS E	21.5	153.0	0.95	0.88	1.05	21.5
North	: Castle	ereagh Road	d (N)											
7	L2	208	1.6	208	1.6	0.158	7.9	LOS A	2.4	17.1	0.26	0.63	0.26	48.0
8	T1	1405	2.0	1405	2.0	0.667	40.5	LOS C	26.8	190.5	0.91	0.80	0.91	26.4
9	R2	667	2.0	667	2.0	0.797	38.6	LOS C	14.3	101.7	1.00	0.89	1.08	36.9
Appro	oach	2281	2.0	2281	2.0	0.797	37.0	LOS C	26.8	190.5	0.87	0.81	0.90	31.4
West	: High S	Street (W)												
10	L2	654	2.0	654	2.0	0.373	18.9	LOS B	8.8	62.4	0.68	0.75	0.68	45.4
11	T1	277	1.9	277	1.9	0.343	52.4	LOS D	8.1	57.5	0.90	0.75	0.90	22.8
12	R2	199	1.6	199	1.6	0.421	67.4	LOS E	6.4	45.4	0.97	0.78	0.97	19.2
Appro	bach	1129	1.9	1129	1.9	0.421	35.6	LOS C	8.8	62.4	0.78	0.76	0.78	34.5
All Ve	hicles	5914	1.9	5903 ^{N1}	2.0	0.808	45.0	LOS D	26.8	190.5	0.89	0.83	0.93	28.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pe	destrians						
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P11	South Stage 1	53	52.4	LOS E	0.2	0.2	0.87	0.87
P12	South Stage 2	53	59.6	LOS E	0.2	0.2	0.92	0.92

P1S	South Slip/Bypass Lane Crossing	53	10.9	LOS B	0.1	0.1	0.55	0.55
P2	East Full Crossing	53	41.7	LOS E	0.2	0.2	0.77	0.77
P2S	East Slip/Bypass Lane	53	16.6	LOS B	0.1	0.1	0.49	0.49
	Crossing							
P31	North Stage 1	53	55.0	LOS E	0.2	0.2	0.89	0.89
P32	North Stage 2	53	27.5	LOS C	0.1	0.1	0.87	0.87
P3S	North Slip/Bypass Lane	53	0.5	LOS A	0.0	0.0	0.11	0.11
	Crossing							
P4	West Full Crossing	53	57.7	LOS E	0.2	0.2	0.91	0.91
P4S	West Slip/Bypass Lane	53	20.5	LOS C	0.1	0.1	0.71	0.71
	Crossing							
All Peo	destrians	526	34.2	LOS D			0.71	0.71

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: 2 [2. Mulgoa Rd and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Mulgoa Rd and Union Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Mulgo	oa Road (S)												
2	T1	1342	2.0	1342	2.0	0.394	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
3	R2	181	1.8	181	1.8	0.915	70.1	LOS E	7.4	52.8	0.98	1.55	3.19	18.4
Appro	ach	1523	1.9	1523	1.9	0.915	8.4	NA	7.4	52.8	0.12	0.18	0.38	47.1
East:	Union I	Road (E)												
4	L2	249	1.8	247	1.8	0.309	7.5	LOS A	1.6	11.3	0.58	0.76	0.62	46.3
Appro	ach	249	1.8	<mark>247</mark> ^N	¹ 1.8	0.309	7.5	LOS A	1.6	11.3	0.58	0.76	0.62	46.3
North:	Mulgo	a Road (N)												
7	L2	192	1.2	191	1.2	0.104	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	38.5
8	T1	1597	2.0	1596	2.0	0.276	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	1788	1.9	<mark>1787</mark> N	¹ 1.9	0.276	0.6	NA	0.0	0.0	0.00	0.06	0.00	59.0
All Ve	hicles	3561	1.9	<mark>3557</mark> N	¹ 1.9	0.915	4.4	NA	7.4	52.8	0.09	0.16	0.21	52.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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V Site: 3 [3. High St and Civic Roundabout]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

High and Civic Roundabout East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Roundabout

Mov	Novement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival I Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rate		km/h
South	n: Link F	Rd (S)												
1	L2	52	0.0	50	0.0	0.204	5.7	LOS A	0.7	5.1	0.67	0.74	0.67	23.1
2	T1	2	0.0	2	0.0	0.204	5.6	LOS A	0.7	5.1	0.67	0.74	0.67	44.7
3	R2	52	0.0	50	0.0	0.204	10.7	LOS A	0.7	5.1	0.67	0.74	0.67	23.1
Appro	oach	105	0.0	<mark>101</mark> N1	0.0	0.204	8.2	LOS A	0.7	5.1	0.67	0.74	0.67	24.4
East:	High S	t (E)												
5	T1	1000	1.9	991	1.9	0.472	2.5	LOS A	2.3	16.3	0.21	0.30	0.21	40.4
6	R2	29	0.0	29	0.0	0.472	7.8	LOS A	2.3	16.3	0.22	0.30	0.22	50.4
Appro	bach	1029	1.9	<mark>1020</mark> N1	1.9	0.472	2.6	LOS A	2.3	16.3	0.21	0.30	0.21	41.1
North	: Civic	PI (N)												
7	L2	53	2.0	53	2.0	0.171	3.9	LOS A	0.7	4.9	0.46	0.65	0.46	42.0
9	R2	100	2.0	100	2.0	0.171	9.1	LOS A	0.7	4.9	0.46	0.65	0.46	42.0
Appro	bach	153	2.0	153	2.0	0.171	7.3	LOS A	0.7	4.9	0.46	0.65	0.46	42.0
West	: High S	St (W)												
10	L2	74	2.0	74	2.0	0.253	2.8	LOS A	1.3	9.0	0.18	0.28	0.18	47.5
11	T1	463	2.0	463	2.0	0.253	2.3	LOS A	1.3	9.0	0.18	0.28	0.18	39.1
Appro	bach	537	2.0	537	2.0	0.253	2.4	LOS A	1.3	9.0	0.18	0.28	0.18	42.0
All Ve	ehicles	1824	1.8	1811 ^{N1}	1.8	0.472	3.3	LOS A	2.3	16.3	0.25	0.35	0.25	40.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 4 [4. High St and Worth St]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

High and Worth East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Cycle Time)

Move	ovement Performance - Vehicles													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Iotai	ΗV	Iotal	ΗV	Sath	Delay	Service	venicies	Distance	Queued	Stop Rate	Cycles S	peed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Worth	n St (S)												
1	L2	215	2.0	205	2.0	0.837	65.9	LOS E	13.0	92.2	1.00	0.94	1.24	3.8
2	T1	114	2.0	109	2.0	0.505	50.2	LOS D	7.7	54.6	0.96	0.78	0.96	22.5
3	R2	34	0.0	32	0.0	0.505	54.4	LOS D	7.7	54.6	0.96	0.78	0.96	22.4
Appro	bach	362	1.8	346 ^{N1}	1.8	0.837	59.9	LOS E	13.0	92.2	0.98	0.88	1.13	12.6
East:	High S	t (E)												
4	L2	81	1.9	81	1.9	0.455	26.5	LOS B	12.7	90.0	0.71	0.66	0.71	30.8
5	T1	668	1.9	668	1.9	0.455	21.9	LOS B	16.1	114.6	0.71	0.64	0.71	31.3
6	R2	211	2.0	211	2.0	0.419	28.8	LOS C	8.4	59.8	0.72	0.77	0.72	35.6
Appro	bach	960	1.9	960	1.9	0.455	23.8	LOS B	16.1	114.6	0.71	0.67	0.71	32.6
North	: Worth	St (N)												
7	L2	1	0.0	1	0.0	0.497	33.6	LOS C	8.8	62.6	0.79	0.66	0.79	35.9
8	T1	194	1.9	194	1.9	0.497	29.1	LOS C	8.8	62.6	0.79	0.66	0.79	28.2
9	R2	309	2.0	309	2.0	0.710	34.9	LOS C	13.1	93.4	0.97	0.84	0.99	25.9
Appro	bach	504	2.0	504	2.0	0.710	32.6	LOS C	13.1	93.4	0.90	0.77	0.91	26.8
West:	High S	St (W)												
10	L2	203	2.0	203	2.0	0.404	35.7	LOS C	8.9	63.7	0.80	0.78	0.80	29.0
11	T1	279	1.8	278	1.8	0.150	18.2	LOS B	4.5	31.6	0.59	0.49	0.59	36.4
12	R2	286	1.6	286	1.6	1.139	211.4	LOS F	36.8	261.1	1.00	1.53	2.38	2.7
Appro	bach	768	1.8	<mark>767</mark> ^{N1}	1.8	1.139	94.8	LOS F	36.8	261.1	0.80	0.95	1.31	13.4
All Ve	hicles	2595	1.9	2577 ^{N1}	1.9	1.139	51.5	LOS D	36.8	261.1	0.81	0.80	0.99	20.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m								
P1	South Full Crossing	53	21.1	LOS C	0.1	0.1	0.59	0.59						
P2	East Full Crossing	53	29.5	LOS C	0.1	0.1	0.70	0.70						

P3 P3S	North Full Crossing North Slip/Bypass Lane Crossing	53 53	22.3 17.1	LOS C LOS B	0.1 0.1	0.1 0.1	0.61 0.53	0.61 0.53
P4 P4S	West Full Crossing West Slip/Bypass Lane Crossing	53 53	54.3 40.1	LOS E LOS E	0.2 0.1	0.2 0.1	0.95 0.82	0.95 0.82
All Peo	destrians	316	30.7	LOS D			0.70	0.70

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 5 [5. Worth St and Union Ln]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Worth St and Union Ln East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Worth	n St (S)												
1	L2	75	1.5	71	1.5	0.039	3.9	LOS A	0.0	0.0	0.00	0.52	0.00	27.5
2	T1	321	2.0	305	2.0	0.159	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
Appro	ach	396	1.9	<mark>376</mark> ^{N1}	¹ 1.9	0.159	0.7	NA	0.0	0.0	0.00	0.10	0.00	42.9
East:	Union I	_n (E)												
4	L2	18	0.0	18	0.0	0.111	4.7	LOS A	0.3	2.2	0.52	0.63	0.52	34.2
5	T1	26	0.0	26	0.0	0.111	9.9	LOS A	0.3	2.2	0.52	0.63	0.52	34.2
6	R2	25	0.0	25	0.0	0.120	11.3	LOS A	0.2	1.7	0.63	0.81	0.63	32.1
Appro	ach	69	0.0	69	0.0	0.120	9.1	LOS A	0.3	2.2	0.56	0.70	0.56	33.4
North	: Worth	St (N)												
8	T1	437	1.7	415	1.8	0.160	0.4	LOS A	13.8	97.9	0.07	0.08	0.07	41.9
9	R2	116	1.5	112	1.5	0.160	6.1	LOS A	0.8	5.6	0.29	0.31	0.29	28.6
Appro	ach	553	1.7	<mark>527</mark> N1	¹ 1.7	0.160	1.6	NA	13.8	97.9	0.12	0.13	0.12	38.1
All Ve	hicles	1018	1.7	<mark>973</mark> N1	¹ 1.7	0.160	1.8	NA	13.8	97.9	0.11	0.16	0.11	37.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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Site: 6 [6. Worth St and Union Rd]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Worth St and Union Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Signals - Fixed Time Isolated Cycle Time = 130 seconds (Site User-Given Cycle Time)

Move	lovement Performance - Vehicles													
Mov	Turn	Demand F	lows	Arrival F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Iotal	ΗV	Iotal	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		i tato		km/h
South	n: Worth	n St (S)												
1	L2	16	0.0	16	0.0	0.329	68.2	LOS E	3.0	21.2	0.98	0.74	0.98	18.1
2	T1	28	0.0	28	0.0	0.329	63.6	LOS E	3.0	21.2	0.98	0.74	0.98	18.1
3	R2	4	0.0	4	0.0	0.329	68.2	LOS E	3.0	21.2	0.98	0.74	0.98	26.1
Appro	bach	48	0.0	48	0.0	0.329	65.5	LOS E	3.0	21.2	0.98	0.74	0.98	19.0
East:	Union I	Rd (E)												
4	L2	33	0.0	33	0.0	0.188	22.3	LOS B	6.0	42.5	0.57	0.52	0.57	39.6
5	T1	147	2.0	147	2.0	0.188	17.8	LOS B	6.0	42.5	0.57	0.52	0.57	33.3
6	R2	359	1.9	359	1.9	1.080	168.1	LOS F	44.4	315.6	1.00	1.35	1.99	8.9
Appro	bach	539	1.8	539	1.8	1.080	118.1	LOS F	44.4	315.6	0.86	1.07	1.52	12.3
North	: Worth	St (N)												
7	L2	421	2.0	406	2.0	0.925	75.2	LOS F	11.5	81.6	1.00	1.02	1.31	17.4
8	T1	32	0.0	30	0.0	0.173	39.7	LOS C	4.1	28.8	0.81	0.71	0.81	24.5
9	R2	59	0.0	54	0.0	0.173	43.6	LOS D	4.1	28.8	0.81	0.71	0.81	5.0
Appro	bach	512	1.6	490 ^{N1}	1.7	0.925	69.5	LOS E	11.5	81.6	0.97	0.96	1.23	17.2
West	Union	Rd (W)												
10	L2	163	1.9	163	1.9	0.156	13.4	LOS A	4.3	30.7	0.42	0.63	0.42	13.3
11	T1	339	2.0	338	2.0	0.479	19.3	LOS B	11.5	81.6	0.63	0.56	0.63	33.8
12	R2	12	0.0	12	0.0	0.479	23.6	LOS B	11.5	81.6	0.64	0.56	0.64	32.9
Appro	bach	514	1.9	<mark>513</mark> ^{N1}	1.9	0.479	17.5	LOS B	11.5	81.6	0.56	0.58	0.56	31.3
All Ve	hicles	1613	1.7	1590 ^{N1}	1.8	1.080	69.1	LOS E	44.4	315.6	0.80	0.87	1.10	16.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m								
P1	South Full Crossing	53	17.8	LOS B	0.1	0.1	0.52	0.52						
P2	East Full Crossing	53	41.7	LOS E	0.2	0.2	0.80	0.80						

P3	North Full Crossing	53	20.5	LOS C	0.1	0.1	0.56	0.56
P4	West Full Crossing	53	59.3	LOS E	0.2	0.2	0.96	0.96
All Pedestrians		211	34.8	LOS D			0.71	0.71

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 24 June 2020 9:43:46 AM Project: Z:\PCI - PROJECT WORK FILES\NSW\TOGA - PENRITH\SIDRA Model\S4.55 East Scheme - Response to TfNSW RFIs\2026 Development PM Peak\200623 - East DA Scheme - 2026 Roundabout, Development, Link - PM Peak One-Way Link (FSR 6-1 Volumes).sip8
🏧 Site: 7 [7. Union Rd and Link Rd]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Union Rd and Link Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	Union	Rd (E)												
5	T1	183	1.8	182	1.8	0.122	0.4	LOS A	0.3	1.8	0.13	0.09	0.13	39.3
6	R2	38	0.0	35	0.0	0.122	5.3	LOS A	0.3	1.8	0.13	0.09	0.13	39.3
Appro	ach	221	1.5	216 ^{N1}	1.5	0.122	1.2	NA	0.3	1.8	0.13	0.09	0.13	39.3
North:	Link F	Rd (N)												
7	L2	33	0.0	32	0.0	0.181	8.0	LOS A	0.5	3.5	0.46	0.95	0.46	17.7
9	R2	75	1.6	72	1.6	0.181	9.5	LOS A	0.5	3.5	0.46	0.95	0.46	17.7
Appro	ach	107	1.1	<mark>104</mark> ^{N1}	1.1	0.181	9.0	LOS A	0.5	3.5	0.46	0.95	0.46	17.7
West:	Union	Rd (W)												
10	L2	79	0.0	79	0.0	0.192	3.9	LOS A	0.8	5.9	0.00	0.12	0.00	42.0
11	T1	288	1.9	288	1.9	0.192	0.0	LOS A	0.8	5.9	0.00	0.12	0.00	42.0
Appro	ach	367	1.5	367	1.5	0.192	0.8	NA	0.8	5.9	0.00	0.12	0.00	42.0
All Ve	hicles	696	1.5	<mark>687</mark> N1	1.5	0.192	2.2	NA	0.8	5.9	0.11	0.23	0.11	34.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

V Site: 8 [8. Union Ln and Link Rd]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Union Ln and Link Rd East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles														
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h	
South	: Link	Road (S)													
2	T1	16	0.0	16	0.0	0.008	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0	
Appro	bach	16	0.0	16	0.0	0.008	0.0	NA	0.0	0.0	0.00	0.00	0.00	50.0	
East:	Union	Ln (E)													
4	L2	91	1.9	87	1.9	0.109	3.9	LOS A	0.4	2.9	0.06	0.53	0.06	25.9	
6	R2	91	1.9	87	1.9	0.109	4.3	LOS A	0.4	2.9	0.06	0.53	0.06	25.9	
Appro	bach	181	1.9	<mark>174</mark> ^{N1}	1.9	0.109	4.1	LOS A	0.4	2.9	0.06	0.53	0.06	25.9	
All Ve	hicles	197	1.7	<mark>190</mark> ^{N1}	1.8	0.109	3.7	NA	0.4	2.9	0.05	0.48	0.05	26.4	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

🎟 Site: Drwy1 [Driveway 1]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Driveway 1 East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand I Total	lows HV	Arrival F Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Link l	Rd (S)												
1	L2	128	0.0	125	0.0	0.068	7.5	LOS A	0.0	0.0	0.00	0.79	0.00	20.7
2	T1	1	0.0	1	0.0	0.068	0.0	LOS A	0.0	0.0	0.00	0.79	0.00	19.5
Appro	ach	129	0.0	126 ^{N1}	0.0	0.068	7.4	NA	0.0	0.0	0.00	0.79	0.00	20.7
North:	: Link F	Rd (N)												
8	T1	84	2.0	80	2.0	0.042	0.0	LOS A	0.0	0.0	0.01	0.01	0.01	48.5
9	R2	1	0.0	1	0.0	0.042	4.4	LOS A	0.0	0.0	0.01	0.01	0.01	14.0
Appro	ach	85	2.0	<mark>81</mark> ^{N1}	2.0	0.042	0.1	NA	0.0	0.0	0.01	0.01	0.01	44.8
West:	Drivev	vay 1												
10	L2	16	0.0	16	0.0	0.027	2.3	LOS A	0.1	0.7	0.00	1.00	0.00	9.0
12	R2	16	0.0	16	0.0	0.027	2.8	LOS A	0.1	0.7	0.00	1.00	0.00	9.0
Appro	ach	32	0.0	32	0.0	0.027	2.5	LOS A	0.1	0.7	0.00	1.00	0.00	9.0
All Ve	hicles	246	0.7	239 ^{N1}	0.7	0.068	4.3	NA	0.1	0.7	0.00	0.55	0.00	19.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

🎟 Site: Drwy2 [Driveway 2]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Driveway 2 East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total	Flows HV	Arrival I Total	lows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	Union	Rd (E)												
5	T1	226	2.0	224	2.0	0.120	0.0	LOS A	0.0	0.3	0.02	0.02	0.02	47.7
6	R2	5	0.0	5	0.0	0.120	7.0	LOS A	0.0	0.3	0.02	0.02	0.02	17.6
Appro	ach	232	2.0	<mark>229</mark> ^{N1}	2.0	0.120	0.2	NA	0.0	0.3	0.02	0.02	0.02	44.7
North:	: Drive	way 2												
7	L2	3	0.0	3	0.0	0.014	3.3	LOS A	0.0	0.3	0.42	0.87	0.42	8.5
9	R2	7	0.0	7	0.0	0.014	4.9	LOS A	0.0	0.3	0.42	0.87	0.42	8.5
Appro	ach	11	0.0	11	0.0	0.014	4.5	LOS A	0.0	0.3	0.42	0.87	0.42	8.5
West:	Union	Rd (W)												
10	L2	14	0.0	14	0.0	0.150	7.5	LOS A	0.0	0.0	0.00	0.06	0.00	25.9
11	T1	275	2.0	275	2.0	0.150	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	44.5
Appro	ach	288	1.9	288	1.9	0.150	0.4	NA	0.0	0.0	0.00	0.06	0.00	42.0
All Ve	hicles	531	1.9	<mark>528</mark> N1	1.9	0.150	0.4	NA	0.0	0.3	0.02	0.06	0.02	39.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Site: UrbApart [Urban Apartments]

♦♦ Network: N101 [Network Model - 2026 Development PM Peak]

Urban Apartments East DA Scheme (FSR 6:1) One-Way Link Northbound to High/ Civic Roundabout Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles														
Mov ID	Turn	Demand F Total	lows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	verage Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h	
East:	Union	Lane (E)													
5	T1	167	2.0	161	2.0	0.110	0.0	LOS A	0.0	0.0	0.00	0.22	0.00	37.8	
6	R2	51	0.0	50	0.0	0.110	5.8	LOS A	0.0	0.0	0.00	0.22	0.00	45.7	
Appro	bach	218	1.5	<mark>210</mark> N1	1.5	0.110	1.4	NA	0.0	0.0	0.00	0.22	0.00	43.4	
North	: Urbar	Apartment	Acces	ss											
9	R2	14	0.0	14	0.0	0.013	3.0	LOS A	0.0	0.3	0.26	0.87	0.26	9.9	
Appro	bach	14	0.0	14	0.0	0.013	3.0	LOS A	0.0	0.3	0.26	0.87	0.26	9.9	
All Ve	hicles	232	1.4	<mark>224</mark> N1	1.5	0.110	1.5	NA	0.0	0.3	0.02	0.26	0.02	28.3	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.